

10 VICINITY MAP

SCALE: N.T.S.



# MADERA UNIFIED SCHOOL DISTRICT

1205 S. Madera Ave. Madera, California 93637  
(559) 675-4548



11 SITE PLAN

SCALE: N.T.S.

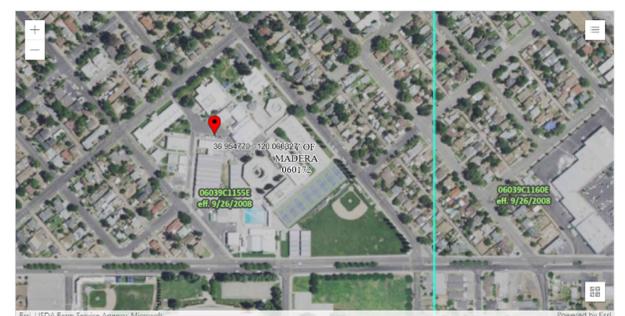
<b>OWNER:</b>	MADERA UNIFIED SCHOOL DISTRICT 1205 S. MADERA AVE. MADERA, CA 93637 T: (559) 675-4548	
<b>STRUCTURAL:</b>	BROOKS RANSOM ASSOCIATES 7415 N. PALM AVE. STE. 100 FRESNO, CA 93711 T: (559) 446-9444 F: (559) 444-8404 CONTACT: ARTURO LOPEZ	
<b>ELECTRICAL:</b>	HARDIN-DAVIDSON ENGINEERING 356 POLLASKY AVE. STE. 200 CLOVIS, CA 93612 T: (559) 323-4995	

9 CONSULTANTS

SCALE: N.T.S.

FLOOD HAZARD ZONE INFORMATION:

- FLOOD ZONE DESIGNATION: ZONE X - OTHER AREAS OUTSIDE OF THE 0.2% ANNUAL CHANGE FLOODPLAIN
- FIRM PANEL DESIGNATION: MAP# 06039C1155E
- FIRM EFFECTIVE DATE: SEPTEMBER 26, 2008
- BASE FLOOD ELEVATION: N/A



10 FLOOD ZONE

SCALE: N.T.S.

PROJECT INFORMATION:

- PROJECT NAME:** MADERA SOUTH HIGH SCHOOL
- LOCATION:** 705 N. PECAN AVE. MADERA, CALIFORNIA 93637
- PROJECT DESCRIPTION:** PROVIDE NEW MODEL NEVCO 1606-PC OUTDOOR BASEBALL SCOREBOARD

SCOPE OF WORK:

- CONSTRUCT SCOREBOARDS STRUCTURAL SUPPORTS AND FOOTINGS.
- MAKE ELECTRICAL POWER CONNECTION FROM EXISTING ELECTRICAL PULL BOX, INCLUDING DISCONNECT.
- PAINT EXPOSED STEEL.

GENERAL NOTES:

- ALL WORK SHALL BE DONE IN ACCORDANCE WITH THESE CONSTRUCTIONS DRAWINGS, THE CONTRACT SPECIFICATIONS AND, WHERE APPLICABLE, THE CITY OF MADERA AND THE STATE OF CALIFORNIA STANDARD SPECIFICATIONS.
- THE CONTRACTOR SHALL COORDINATE ALL CONSTRUCTION ACTIVITIES WITH THE SCHOOL DISTRICT'S USE OF THE FACILITIES AND OTHER CONTRACTORS WHO MAY BE DOING CONSTRUCTION WITHIN THE PROJECT SITE.
- THE CONTRACTORS SHALL CONTACT DISTRICT OFFICIALS FOR DETERMINATION OF DEPTH AND LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION IN THE PROJECT SITE.
- BEFORE COMMENCING WORK, THE CONTRACTOR SHALL NOTIFY ALL UTILITY AUTHORITIES OR UTILITY COMPANIES HAVING POSSIBLE INTEREST IN THE WORK OF THE CONTRACTOR'S INTENTION TO EXCAVATE PROXIMATE TO EXISTING FACILITIES AND THE CONTRACTOR SHALL VERIFY THE LOCATION OF ANY UTILITIES IN THE WORK AREA, NOTIFY U.S.A. AT (800) 642-2444, TWO (2) DAYS PRIOR TO EXCAVATION.
- CONTRACTOR SHALL PROVIDE 6' HIGH TEMPORARY CHAIN LINK FENCE AROUND THE PERIMETER OF THE WORK AREAS EXCEPT WHERE ENCLOSED BY EXISTING FENCING.
- ALL CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 CALIFORNIA BUILDING CODE (CBC).
- CHANGE TO THE APPROVED DRAWINGS SHALL BE MADE BY ADDENDA OR CONSTRUCTION CHANGE DOCUMENT (CCD) APPROVED BY DSA, AS REQUIRED BY SECTION 4-398, PART 1, TITLE 24, CCR.
- A "DSA CERTIFIED" CLASS 3 PROJECT INSPECTOR EMPLOYED BY THE DISTRICT (OWNER) AND APPROVED BY DSA SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK. THE DUTIES OF THE INSPECTOR ARE DEFINED IN SECTION 4-342, PART 1, TITLE 24, CCR.
- A DSA ACCEPTED TESTING LABORATORY DIRECTLY EMPLOYED BY THE DISTRICT (OWNER) SHALL CONDUCT ALL THE REQUIRED TESTS AND INSPECTIONS FOR THE PROJECT.
- FIRE SAFETY DURING DEMOLITION AND CONSTRUCTION SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF CHAPTER 33 OF THE 2022 CALIFORNIA BUILDING CODE AND THE APPLICABLE PROVISIONS OF CHAPTER 33 OF THE CALIFORNIA FIRE CODE.
- THE INTENT OF THESE DRAWINGS AND SPECIFICATIONS IS THAT THE WORK OF THE ALTERATION, REHABILITATION OR RECONSTRUCTION IS TO BE IN ACCORDANCE WITH TITLE 24, CCR. SHOULD ANY EXISTING CONDITIONS SUCH AS DETERIORATION OR NON-COMPLYING CONSTRUCTION BE DISCOVERED WHICH IS NOT COVERED BY THE CONTRACT DOCUMENTS WHEREIN THE FINISHED WORK WILL NOT COMPLY WITH TITLE 24, CCR, A CONSTRUCTION CHANGE DOCUMENT (CCD), OR A SEPARATE SET OF PLANS AND SPECIFICATIONS, DETAILING AND SPECIFYING THE REQUIRED WORK SHALL BE SUBMITTED TO AND APPROVED BY DSA BEFORE PROCEEDING WITH THE WORK. (SECTION 4-311(C), PART 1, TITLE 24, CCR)
- GRADING PLANS, DRAINAGE IMPROVEMENTS, ROAD AND ACCESS REQUIREMENTS AND ENVIRONMENTAL HEALTH CONSIDERATIONS SHALL COMPLY WITH ALL LOCAL ORDINANCES.
- SUBSTITUTIONS AFFECTING DSA REGULATED ITEMS SHALL BE CONSIDERED AS A CONSTRUCTION CHANGE DOCUMENT OR ADDENDUM, AND SHALL BE APPROVED BY DSA PRIOR TO FABRICATION AND INSTALLATION PER DSA IR A-6 AND SECTION 338(C) PART 1, TITLE 24 CCR.

6 GENERAL DESIGN NOTES

SCALE: N.T.S.

SHEET INDEX:

SHT. NO.	TITLE
<b>GENERAL</b>	
C-1	COVER SHEET
SP-1	STRUCTURAL PARTIAL SITE PLAN
<b>NEVCO DSA P.C. 04-122317</b>	
SB0.1	COVER SHEET
SB0.2	STRUCTURAL NOTES
SB0.3	EXAMPLE DSA 103-TESTING AND INSPECTIONS
SB1.1	MARQUEE CAISSON - EMBEDDED
SB1.2	MARQUEE CAISSON - BOLTED
SB1.3	MARQUEE MAT FOOTING
SB2.1	TWO COLUMN CAISSON - EMBEDDED
SB2.2	TWO COLUMN CAISSON - BOLTED
SB2.3	TWO COLUMN MAT FOOTING
SB3.1	THREE COLUMN CAISSON - EMBEDDED
SB3.2	THREE COLUMN CAISSON - BOLTED
SB3.3	THREE COLUMN MAT FOOTING
SB4.1	FOUR COLUMN CAISSON - EMBEDDED
SB4.2	FOUR COLUMN CAISSON - BOLTED
SB4.3	FOUR COLUMN MAT FOOTING
SB5.1	ATTACHMENT DETAILS
SB5.2	OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS
SB5.3	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS
SB5.4	DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS 4-10mm VIDEO BOARD
SB6.1	INDOOR WALL MOUNTED SCOREBOARD

<b>ELECTRICAL</b>	
E1	ELECTRICAL SYMBOLS, NOTES AND DETAILS
E2	ELECTRICAL PARTIAL SITE PLAN
TOTAL SHEET COUNT: 24	

3 SHEET INDEX

SCALE: N.T.S.

NOT APPLICABLE

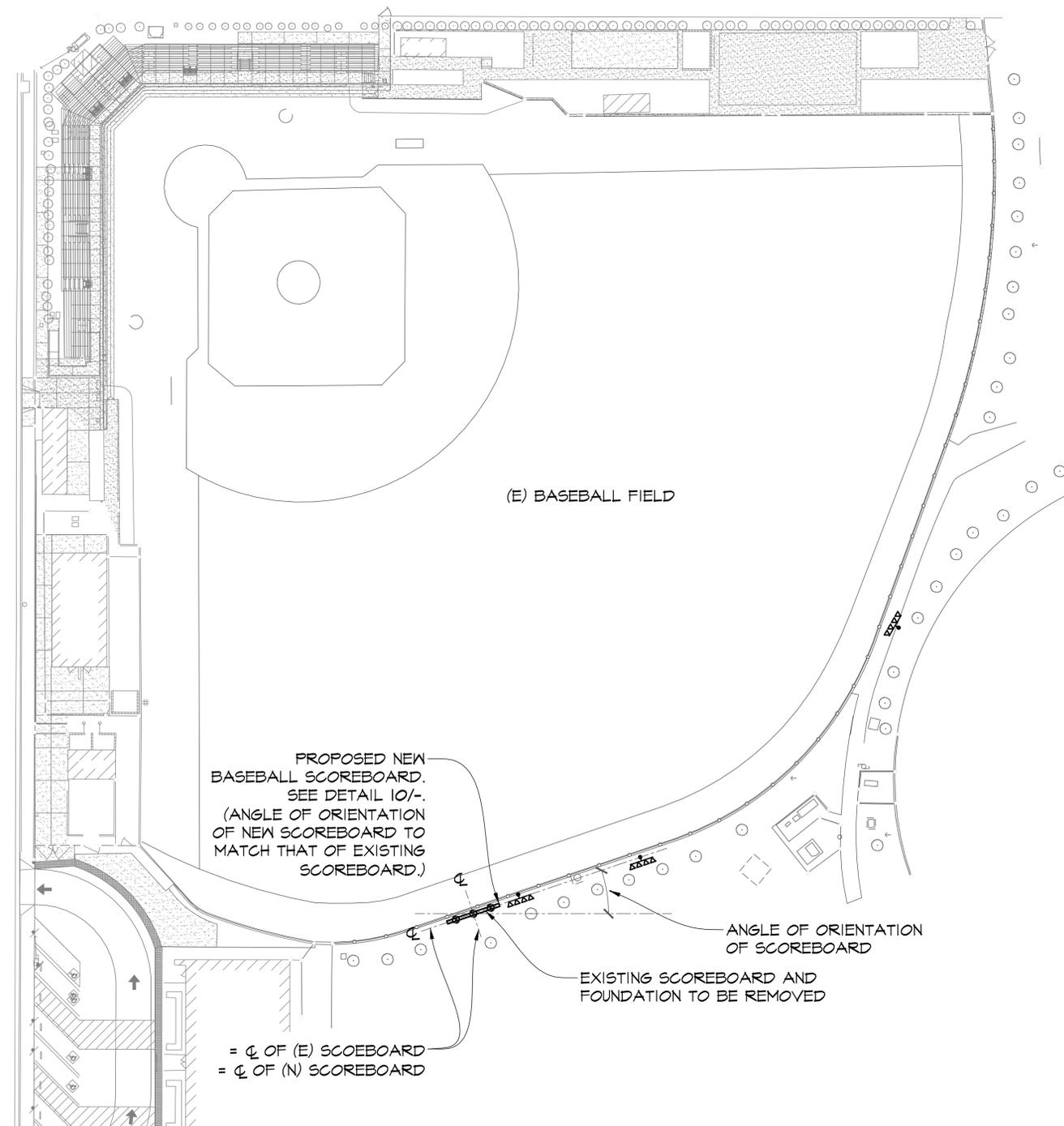
4 DEFERRED SUBMITTAL

SCALE: N.T.S.



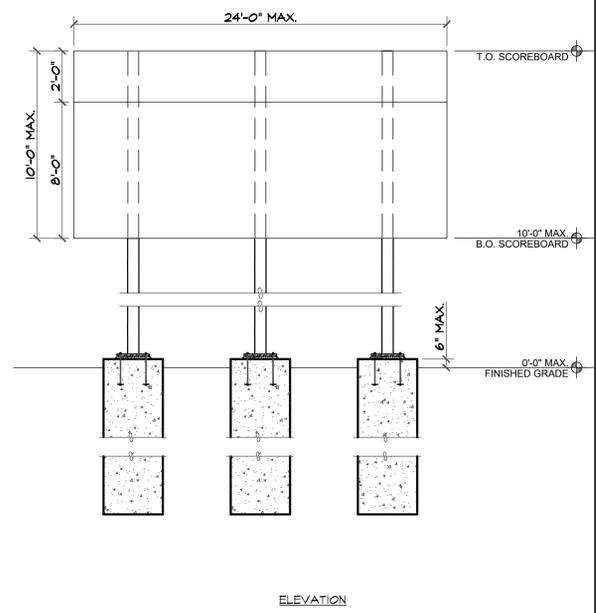
12 AREA OF WORK

SCALE: N.T.S.



SEE DSA P.C. 04-122817 FOR NEVCO 1606-PC SCOREBOARD

AT BAT	BALL	STRIKE	OUT	H	E								
33	8	2	2	8	8								
NEVCO													
GUESTS	1	2	3	4	5	6	7	8	9	10	RUNS	HITS	PITCHES
HOME	0	0	0	0	0	0	0	0	0	0	6	8	189
	2	0	0	0	0	0	0	0	0	0	10	13	104



10 DETAIL

SCALE: N.T.

**PARTIAL SITE PLAN**  
MADERA HIGH SCHOOL  
**BASEBALL SCOREBOARD**  
MADERA, CA 93637

REVISIONS


**BrooksRansom**  
ASSOCIATES  
7415 N. PALM AVE. STE. 100 | FRESNO, CA 93711  
(559) 448-8444 OFFICE | (559) 448-8404 FAX



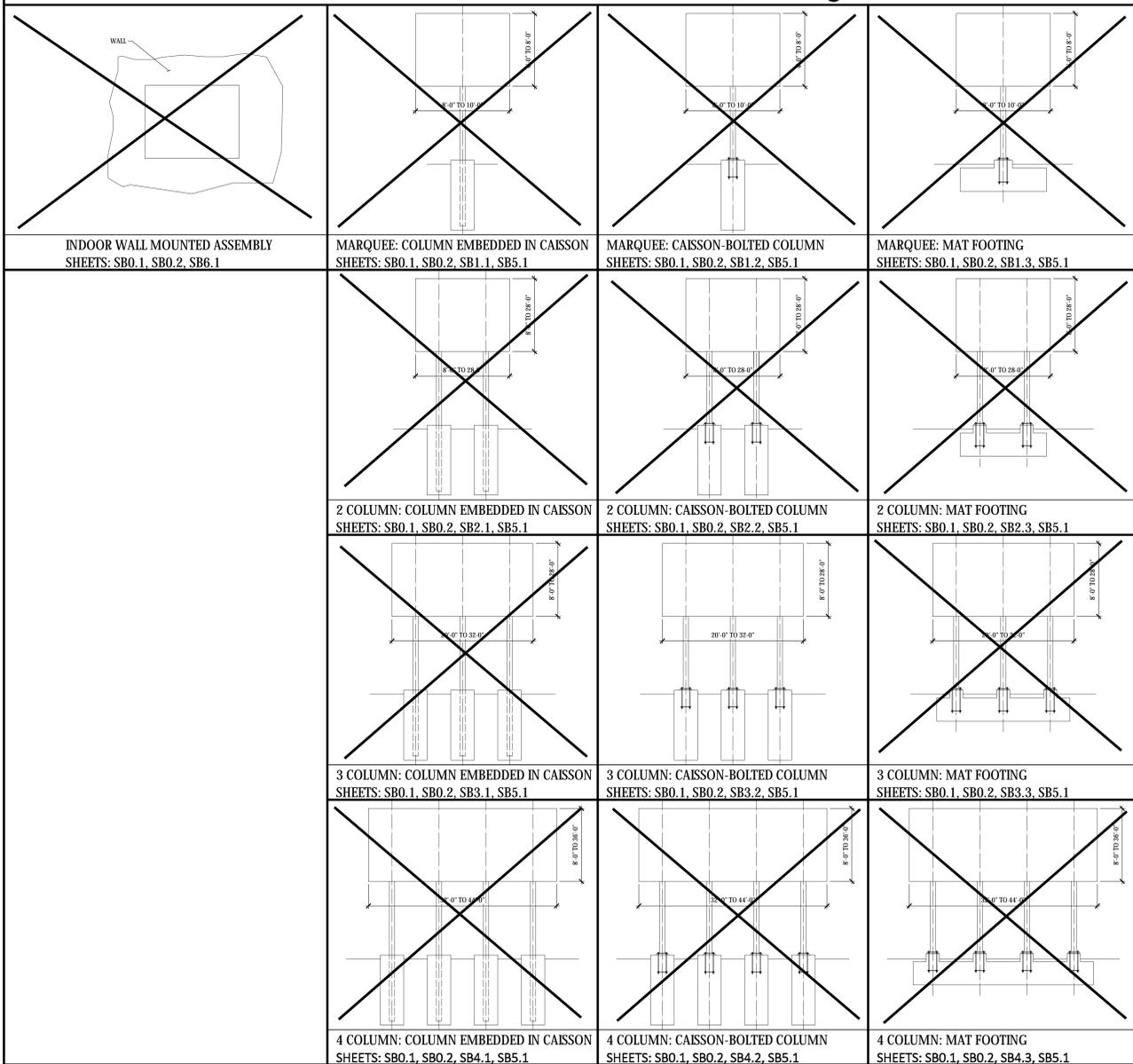


TABLE C - SITE SPECIFIC SEISMIC AND WIND VALUES

EARTHQUAKE DESIGN DATA	MAXIMUM		SITE SPECIFIC	
	S <sub>w</sub>	S <sub>u</sub>	S <sub>w</sub>	S <sub>u</sub>
Mapped Spectral Response Accelerations (Maximum)	3.73 g	0.71 g	0.71 g	0.24 g
Site Class	D	D	D	D
Spectral Response Coefficients (Maximum)	2.49 g	0.62 g	0.62 g	0.42 g
Wind Design Data	Value	Value	Value	Value
Design Wind Speed (3-sec gust), V <sub>100</sub>	100 mph	94 mph	94 mph	94 mph
Exposure Category	C	C	C	C

TABLE D - SITE FLOOD ZONE  
THIS SECTION NOT REQUIRED IF SITE IS IN FLOOD ZONE X

Geotechnical Engineer:	
Letter Dated:	

TABLE B - STRUCTURAL DESIGN VALUES

All values reported are unfactored and strength level, unless noted otherwise		Value
<b>Gravity Design Data</b>		
Dead Loads:		
Sign Dead Load		PER SCHEDULE
Snow Loads:		
Ground Snow Load, P <sub>s</sub> (Maximum)		30 psf
<b>Deflection Criteria:</b>		
Sign, Wind Load		H/240
<b>Wind Design Data</b>		
Design Wind Speed (3-sec gust), V <sub>100</sub>		100 mph
Design Wind Speed (3-sec gust), V <sub>60</sub>		77 mph
Risk Category		II
Exposure Category		C
Applicable Internal Pressure Coefficient		+0.18
Design Wind Pressure(s) for Components & Cladding (Not specifically designed by the Registered Design Professional, and to be modified by applicable factors per ASCE 7)		q = 21.8 k/psf K <sub>A</sub> VARIES
<b>Earthquake Design Data</b>		
Risk Category		II
Importance Factor, I		1.0
Mapped Spectral Response Accelerations (Maximum)		S <sub>w</sub> = 3.73 g S <sub>u</sub> = 1.0 g
Site Class		A through E
Spectral Response Coefficients (Maximum)		S <sub>w</sub> = 2.49 g S <sub>u</sub> = 1.0 g
Seismic Design Category		E
Analysis Procedure Used	Equivalent Lateral Force Procedure (ASCE 7, 12.8)	
<b>Flood Design</b>		
When the scoreboard is located in a flood zone other than Zone X, a letter stamped and signed from a Geotechnical Engineer is needed to validate allowable soil values specified in the PC are still applicable.		
<b>Geotechnical Design Data</b>		
Geotechnical Design Based on:		Value
2022 California Building Code, Chapter 18A, Table 1806.A.2 (Class 5 Material)		
Allowable Soil Bearing Pressure (DL + LL)		1,500 psf
Design Passive Pressure, P <sub>s</sub> (Tabular value has been increased per CBC Section 1806A.3.4 for pier design)		100 pcf
Design Skin Friction, f <sub>s</sub>		100 psf

SCOREBOARD ASSEMBLY WORKSHEET (TABLE A, C & D) INSTRUCTIONS

- STEP 1: DETERMINE DESIRED SCOREBOARD ASSEMBLY. FILL OUT SCOREBOARD ASSEMBLY TABLE (TABLE A BELOW). PROVIDE NEVCO PART NUMBERS, PART HEIGHT, PART WIDTH, AND PART WEIGHTS.
- STEP 2: DETERMINE TOTAL ASSEMBLY HEIGHT, WIDTH, AND WEIGHT, TABLE A
- STEP 3: BASED ON TOTAL ASSEMBLY WIDTH, DETERMINE THE NUMBER OF REQUIRED COLUMNS. SEE SHEETS SB1.X FOR 1 COLUMN ASSEMBLY OPTIONS, SB2.X FOR 2 COLUMN ASSEMBLY OPTIONS, SB3.X FOR 3 COLUMN ASSEMBLY OPTIONS, SB4.X FOR 4 COLUMN ASSEMBLY OPTIONS, SB6.1 FOR WALL MOUNTED ASSEMBLY OPTIONS (SKIP STEPS 4, 5, & 7)
- STEP 4: PICK FOUNDATION TYPE (CAISSON WITH EMBEDDED COLUMN, CAISSON WITH BOLTED COLUMN, OR MAT FOOTING). MARK APPLICABLE SHEET ON SHEET INDEX, SB0.1
- STEP 5: MARK APPLICABLE CHECK BOX FOR SCOREBOARD SIZE ON DETAIL 'A' OF SELECTED COLUMN/FOUNDATION OPTION (SHEETS SB1.X, SB2.X, SB3.X OR SB4.X)
- STEP 6: FILL IN SITE SPECIFIC SEISMIC AND WIND VALUES TABLE C ON SB0.1
- STEP 7: FILL IN SITE SPECIFIC FLOOD ZONE AS REQUIRED, TABLE D ON SB0.1
- STEP 8: VERIFY ALL APPLICABLE SHEETS ARE MARKED ON SHEET INDEX, SB0.1. INCLUDE ONLY MARKED SHEETS AS PART OF DSA SUBMITTAL

SITE SPECIFIC SUBMITTAL REQUIREMENTS

- SEE DSA POLICY PL 07-02 FOR ADDITIONAL INSTRUCTIONS REGARDING USE AND APPLICATION OF THIS PRE-CHECK DOCUMENT. ALL SITE SPECIFIC SUBMITTALS SHALL INCLUDE:
- COMPLETED DSA 1 APPLICATION, DSA3, DSA 103, AND FILING FEE AND COPY OF THE PRE-CHECK DOCUMENT WITH APPLICABLE DESIGN OPTION MARKED ON THE MARQUEE, TWO COLUMN, THREE COLUMN, FOUR COLUMN, OR WALL ASSEMBLY SCHEDULES.
  - SITE PLAN OF FACILITY IDENTIFYING ALL STRUCTURES BY DSA APPLICATION NUMBER. LOCATION OF SCOREBOARD SHALL BE IDENTIFIED. ELECTRICAL PANEL SERVING THE SCOREBOARD SHALL BE LOCATED AND IDENTIFIED.
  - WHERE WIRELESS CONTROLLERS ARE NOT SPECIFIED, AN ACCESSIBLE PATH OF TRAVEL AND ACCESSIBLE SEATING FOR THE SCOREBOARD OPERATOR SHALL BE IDENTIFIED AND PROVIDED.
  - PROVIDE AN ELEVATION OF PROPOSED SCOREBOARD IDENTIFYING ALL INSTALLED DISPLAY COMPONENTS, SIGNAGE, TRUSSES, AND ADDITIONAL COMPONENTS IN THE PRE-CHECK DOCUMENT. ALL ELEMENT WEIGHTS SHALL BE SPECIFIED.
  - THE APPLICABLE SHEETS SHALL BE IDENTIFIED BY MARKING APPROPRIATE CHECK BOX ON THIS SHEET.
  - THE APPLICABLE CONFIGURATION SHALL BE IDENTIFIED BY MARKING APPROPRIATE CHECK BOX ON THE 'A' DETAILS ON THE APPLICABLE SHEET.
  - PROVIDE CUT SHEETS OF THE BOARDS, BOXES, AND EQUIPMENT TO BE MOUNTED ON THE STRUCTURE. CUT SHEETS SHALL INCLUDE WEIGHTS AND DIMENSIONS
  - SITE SPECIFIC SEISMIC DESIGN CRITERIA SHALL BE PROVIDED IN THE DRAWINGS.
  - SITE SPECIFIC BASIC DESIGN WINDSPEED AND SITE EXPOSURE SHALL BE PROVIDED ON THE DRAWINGS. SEE TABLE C.
  - STEEL COATING SPECIFICATIONS FOR WEATHER PROTECTION IF DIFFERENT THAN NOTED ON SB0.3

GENERAL NOTES AND MATERIAL SPECIFICATIONS

- GENERAL REQUIREMENTS
- THE ARCHITECT OR PROFESSIONAL ENGINEER IN GENERAL RESPONSIBLE CHARGE SHALL SIGN AND SEAL ALL DRAWINGS AND SPECIFICATIONS PER TITLE 24, PART 1, SECTIONS 4-316(E) AND 4-317 (H).
  - CHANGES TO THE APPROVED DRAWINGS AND SPECIFICATIONS SHALL BE MADE BY ADDENDA, OR CONSTRUCTION CHANGE DOCUMENTS APPROVED BY THE DIVISION OF THE STATE ARCHITECT (DSA), AS REQUIRED BY TITLE 24, PART 1, SECTION 4-338.
  - THE DISTRICT SHALL EMPLOY A CLASS 2 PROJECT INSPECTOR WHEN OVERALL STRUCTURE HEIGHT IS 35 FEET OR GREATER, OTHERWISE A CLASS 3 PROJECT INSPECTOR MAY BE USED. THE PROJECT INSPECTOR SHALL PROVIDE CONTINUOUS INSPECTION OF THE WORK, AND SHALL SUBMIT VERIFIED REPORTS ON A DSA-6 FORM. THE DUTIES OF THE PROJECT INSPECTION ARE DEFINED IN TITLE 24, PART 1, SECTION 4-342.
  - ALL SCOREBOARD CONTROLS SHALL BE FULLY ACCESSIBLE VIA WIRELESS CONTROL OR COMPLETE DESIGN SHALL BE DEMONSTRATED IN THE SITE SPECIFIC APPLICATION.
  - ALL ASSEMBLIES SHALL HAVE ELECTRICAL DISCONNECT PER CEC 600.6 AND BE ELECTRICALLY GROUNDED PER CEC 600.7, SEE DETAIL B/SB5.1
  - IN FLOOD ZONES, LOCATION OF ELECTRICAL ELEMENTS SHALL CONFORM TO ASCE 24, SECTION 7.2 PER DSA PR-14-01 SECTION 1.2.1.
  - SEE PAGE, SB0.2, FOR ALL MATERIAL SPECIFICATIONS AND NOTES.
  - PROJECT DESIGN PROFESSIONAL OF RECORD IS RESPONSIBLE FOR PREPARATION OF THE PROJECT SPECIFIC DSA 103 AND IS RESPONSIBLE FOR ALL SHOP DRAWING AND SUBMITTAL REVIEWS. SEE SB0.3 FOR EXAMPLE DSA 103

TABLE A - SCOREBOARD ASSEMBLY WORKSHEET (1)

Nevco Part No. or Description	Part Height [ft.]	Part Width [ft.]	Part Weight [lb]
1606-PC-W	8	24	570
ADO 24-2	2	24	106
Total			
TOTAL ASSEMBLY DIMENSIONS & WEIGHT (2)			
Total Assembly Height =	10	ft. 0	in.
Total Assembly Width =	24	ft. 0	in.
Total Assembly Weight =	676	lbs.	
Distance from Finish Grade to Bottom of Sign =	10	ft. 0	in.
Total Height = Total Assembly Height + Distance from Finish Grade to Bottom of Sign =	20	ft. 0	in.
SCOREBOARD ASSEMBLY FOOTNOTES			
1. Verify part number, dimensions, and weight with Nevco			
2. See Step 3 of Scoreboard Assembly Worksheet Instructions			

CHECK ALL THAT APPLY

CHECK ALL THAT APPLY	SHEET INDEX
<input checked="" type="checkbox"/> (REQ'D)	SB0.1 COVER SHEET
<input checked="" type="checkbox"/> (REQ'D)	SB0.2 STRUCTURAL NOTES
<input type="checkbox"/>	SB0.3 EXAMPLE DSA 103 - TESTING AND INSPECTIONS
<input type="checkbox"/>	SB1.1 MARQUEE CAISSON - EMBEDDED
<input type="checkbox"/>	SB1.2 MARQUEE CAISSON - BOLTED
<input type="checkbox"/>	SB1.3 MARQUEE MAT FOOTING
<input type="checkbox"/>	SB2.1 TWO COLUMN CAISSON - EMBEDDED
<input type="checkbox"/>	SB2.2 TWO COLUMN CAISSON - BOLTED
<input type="checkbox"/>	SB2.3 TWO COLUMN MAT FOOTING
<input type="checkbox"/>	SB3.1 THREE COLUMN CAISSON - EMBEDDED
<input checked="" type="checkbox"/>	SB3.2 THREE COLUMN CAISSON - BOLTED
<input type="checkbox"/>	SB3.3 THREE COLUMN MAT FOOTING
<input type="checkbox"/>	SB4.1 FOUR COLUMN CAISSON - EMBEDDED
<input type="checkbox"/>	SB4.2 FOUR COLUMN CAISSON - BOLTED
<input type="checkbox"/>	SB4.3 FOUR COLUMN MAT FOOTING
<input checked="" type="checkbox"/>	SB5.1 ATTACHMENT DETAILS
<input checked="" type="checkbox"/>	SB5.2 OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS
<input type="checkbox"/>	SB5.3 DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS
<input type="checkbox"/>	SB5.4 DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS & 10mm VIDEO BOARD
<input type="checkbox"/>	SB6.1 INDOOR WALL MOUNTED SCOREBOARD

CODE INFORMATION

2022 CALIFORNIA BUILDING STANDARDS CODE (TITLE 24, CCR):

2022 ADMINISTRATIVE CODE, PART 1, TITLE 24 CODE OF REGULATIONS (CCR)  
 2022 CALIFORNIA BUILDING CODE VOLUMES 1 & 2, PART 2, TITLE 24 CCR  
 2022 CALIFORNIA ELECTRICAL CODE, PART 3, TITLE 24 CCR  
 2022 CALIFORNIA MECHANICAL CODE, PART 4, TITLE 24 CCR  
 2022 CALIFORNIA PLUMBING CODE, PART 5, TITLE 24 CCR  
 2022 CALIFORNIA ENERGY CODE, PART 6, TITLE 24 CCR  
 2022 CALIFORNIA FIRE CODE, PART 9, TITLE 24 CCR  
 2022 CALIFORNIA GREEN BUILDING STANDARDS CODE, PART 11, TITLE 24 CCR  
 2022 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 CCR

REFERENCED CODE SECTIONS FOR APPLICABLE STANDARDS:  
 2022 CALIFORNIA BUILDING CODE, CHAPTER 35  
 2022 CALIFORNIA FIRE CODE, CHAPTER 80

APPLICATION # 02-122088

IDENTIFICATION STAMP  
 DIV. OF THE STATE ARCHITECT  
 APP: 02-122088 INC.  
 REVIEWED FOR  
 SS  FLS  ACS   
 DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL L. RANOLD  
 No. 5485  
 STRUCTURAL  
 STATE OF CALIFORNIA  
 EXPIRES 08.09.2023

NEVCO

301 East Harris Avenue, Greenville, Illinois 62246  
 Phone: (618) 664-0960  
 www.nevco.com

APPROVED  
 DIV. OF THE STATE ARCHITECT  
 APP: 04-122317 PC  
 REVIEWED FOR  
 SS  FLS  ACS  CG   
 DATE: 09/20/2024

PRE-CHECK (PC) DOCUMENT  
 CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
 CALIFORNIA REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL L. RANOLD  
 No. 52386  
 STRUCTURAL  
 STATE OF CALIFORNIA  
 EXPIRES 03-31-25

COVER SHEET

SHEET INFORMATION  
 DATE: 08.09.2023  
 DRAWN: JMK  
 CHECKED: MEP  
 SSG JOB #: S23109  
 SHEET: SB0.1

## STRUCTURAL NOTES

### GENERAL NOTES

- The following notes, typical details and schedules shall apply to all phases of this project unless otherwise shown or noted.
- Specific notes and details shall take precedence over general notes and typical details.
- All materials and workmanship shall conform to the minimum standards of the 2022 edition Title 24 of the California Building Code (CBC) and such other regulating agencies exercising authority over any portion of the work. The contractor shall have a current copy of the CBC on the job site.
- The "Contract or Construction Documents" shall consist of these notes, details, schedules, plans, and drawings.
- All specifications, including but not limited to materials and products, shall be those put forth in the "Contract or Construction Documents". No substitutions shall be permitted to be used or assumed to be used in the bidding or construction process without written approval by the Structural Engineer of Record.
- The contractor shall examine the "Contract or Construction Documents" and shall notify the Architect or Structural Engineer of Record of any discrepancies he may find before proceeding with the work.
- All information on existing conditions shown on drawings are based on best present knowledge available, but without guarantee of accuracy. The Contractor shall verify and be responsible for all dimensions and conditions at the site and shall notify the Architect or Structural Engineer of Record of any discrepancies between actual site conditions and information shown on or in the "Contract or Construction Documents" before proceeding with work.
- The Contractor shall immediately notify the Architect or Structural Engineer of Record of any condition which in his opinion might endanger the stability of the structure or cause distress of the structure.
- All work shall conform to the best practice prevailing in the various trades comprising work. The Contractor shall be responsible for coordinating the work of all trades.
- These "Contract or Construction Documents" represent the finished structure, and do not indicate the method of construction. The Contractor shall supervise and direct the work and shall be solely responsible for construction means, methods, techniques, sequences and procedures.
- Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
  - Labeling (as required or specified) shall be provided in accordance with CBC Section 1703A.5.
  - Evaluation and follow-up inspection services (as required or specified), shall conform to CBC Section 1703A.6.
- The Contractor shall provide temporary bracing and shoring for all structural members as required for structural stability of the structure during all phases of construction.
- The Contractor shall take all steps necessary to ensure proper alignment of the structure after the installation of all structural and finish materials. This shall include any necessary preloading of the structure to determine final position of the completed work.
- Observation visits to the project site by field representatives of Architect and/or Structural Engineer of Record (support services) shall not include inspections of safety or protective measures, nor construction procedures, techniques or methods. Any support services performed by Architect or Structural Engineer of Record during any phase of construction, shall be distinguished from continuous and detailed inspection services (as required by any regulating governmental agency, e.g. the Authority Having Jurisdiction) provided by others. These support services, whether of material or work, are performed solely for the purpose of assisting in quality control and in achieving conformance with contract documents, but do not guarantee Contractor's performance and shall not be construed as supervision of construction.
- These notes, details, drawings and specifications (Contract or Construction Documents) do not carry necessary provisions for construction safety. These documents and all phases of construction hereby contemplated are to be governed, at all times, by applicable provisions of the current California Occupational Safety and Health Act.
- Where any conflict occurs between the requirements of federal, state and local laws, codes, ordinances, rules and regulations, the most stringent shall govern.
- Written dimensions shall have precedence over scaled dimensions.
- Drawings (notes, schedules, details and plans) shall have precedence over Structural Calculations.
- In the event that certain features of the construction are not fully shown on the drawings or called for in the General Notes or Specifications, then their construction shall be of the same character as for similar conditions that are shown or called for.
- ASTM designation and all standards refer to the latest amendments.
- These structural "Contract or Construction Documents" shall not be modified without prior written approval of the Structural Engineer of Record.
- Only structural working drawings approved by the Division of the State Architect are permitted to be used for construction on this project. All other drawings or documents are obsolete and are not permitted on the job site, nor shall they be used for any construction purposes. Contractors using unapproved drawings or documents are solely responsible for all work not in accordance with the "approved" drawings.
- A Division of the State Architect certified project inspector employed by the District (Owner) and approved by the Division of the State Architect shall provide continuous inspection of the work. The duties of the inspector are defined in Section 4-342, Part 1, Title 24 California Code of Regulations.

### FOUNDATION NOTES

- Basils: See Structural Design Values Chart, Sheet SBO.1 Table B
- Unexpected soil conditions: Allowable values and foundation design are based upon the minimum values provided in Table 1806A.2 of the 2022 California Building Code. See SBO.1 for values
- Excavate to required depths and dimensions (as indicated in drawings), cut square and smooth with firm level bottoms. Care shall be taken not to over-excavate foundation at lower elevation and prevent disturbing of soils around higher elevation.
- Footings shall be poured in neat excavations, without side forms whenever possible.
- Carry all foundations to required depths into compacted fill or natural soil (as per Structural Plans and Details).
- All foundation excavations shall be inspected and approved by the Inspector of Record or Geotechnical Engineer prior to forming and placement of reinforcing or concrete.
- Foundations shall not be poured until all required reinforcing steel, sleeves, inserts, conduits, pipes, etc. and formwork is properly placed and inspected by the Authority having Jurisdiction.
- The sides and bottoms of excavations which are to have concrete contact must be moistened several times just prior to pouring upon them.
- De-water footings, as required, to maintain dry working conditions.

### REINFORCING STEEL

- All reinforcing steel shall be deformed intermediate grade bars conforming to ASTM A615, Grade 60 ( $f_y = 60$  ksi) unless noted otherwise.
- Reinforcing steel shall not be welded, unless specifically noted otherwise.
- To hold reinforcing bars in their true position and prevent displacement, standard tie and anchorage devices must be provided. Placing of reinforcement shall conform to ACI 318-19 Section 26.6.2.
- Shop drawings for fabrication of any reinforcing steel shall be approved by Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record, for their review, prior to fabrication.
- Refer to typical details for minimum splice length and minimum radius of bend of reinforcing steel.
- All reinforcing steel splices shall be staggered 24", unless specifically noted or detailed otherwise.
- All reinforcing bar bends shall be made cold.
- Fabrication, erection and placement of reinforcing steel shall conform to Concrete Reinforcing Steel Institute of Standard Practice.
- Reinforcing steel shall be clean of rust, grease or other material likely to impair bond.

### CONCRETE

- All concrete shall have a minimum ultimate compressive strength ( $f'_c$ ) as outlined below at 28 days. All concrete shall be regular weight (unless specifically noted otherwise).
  - Concrete for footings: 4,500 psi  $w/c = 0.45$  max.
- Maximum Fly Ash content shall be 15%, by weight, of total cementitious materials and shall conform to ASTM C618.
- All concrete work shall comply with CBC Chapter 19A and ACI 318-19 and latest edition of ACI Manual of Concrete Practice.
- Special Inspection (as required or specified) shall conform to CBC Chapter 17A.
- Cement shall be portland cement Type V and shall conform to ASTM C150.

- Aggregates shall conform to ASTM C33, provide aggregates from a single source.
- Water shall conform to ASTM C94 and be potable.
- Where not specifically detailed, the minimum concrete cover on reinforcing steel shall be:
  - Concrete cast against and permanently exposed to earth or weather: 3"
- All reinforcing steel, anchor bolts, dowels, inserts and any other hardware to be set in concrete shall be well secured in position prior to pouring of concrete.
- Vibrate all concrete as it is placed, with a mechanical vibrator operated by experienced personnel. The vibrator shall be used to consolidate the concrete, not transport it. Reinforcing and forms shall not be vibrated.
- Formwork design and removal shall conform to ACI 318-19 Section 26.11. Remove forms in accordance with the following minimum schedule:
  - Side forms of footings: Minimum 48 hours
  - Column and pier forms: 72 hours & 70% of design strength
- Concrete shall not free fall more than six feet. Use tremie, pump or other approved methods.
- Concrete shall be maintained in a moist condition for a minimum of 5 days after placement.
- The Contractor may use concrete admixtures as a construction means and methods to execute "Contract or Construction Documents". Use of admixture is solely the responsibility of the Contractor.
- Mix designs shall be prepared by an approved testing laboratory, signed by a licensed engineer and shall be submitted to the Project Specific Design Professional of Record for approval. SSG is not responsible for review or approval of site specific concrete mix design.
- Only one grade of concrete shall be allowed on project site at any one time
- Concrete strength shall be verified by standard cylinder tests (in accordance with CBC Section 1905A.1.10) made by an approved testing laboratory.
- Concrete placed during hot weather shall conform to ACI 318-19 Section 26.5.5, and ACI 308R-14.
- Conduits and sleeves placed within structural concrete shall not be tied directly to structural reinforcement.
  - 1" concrete cover shall be maintained around all reinforcement.
- No stakes shall be permitted within the footing section.
- Concrete shall reach minimum 75% design strength or cure for 3 days minimum prior to installation of steel columns and scoreboard components.

### DRILLED CAISSON/PIER AND GRADE BEAM NOTES

- Excavations for drilled caissons/pier shall be performed in compliance with local grading codes and ordinances as well as CBC Chapters 18A and 33A.
- Provide Special Inspection in accordance with CBC Section 1705A.8 and Table 1705A.8.
- Excavations for all drilled caissons/piers shall be approved by the Project Geotechnical Engineer or Project Specific Inspector prior to placing of concrete.
- Reinforcement for drilled caissons/pier shall be approved by the Structural Engineer of Record prior to placing in caisson/pier excavation.
- De-water caisson/pier footings and building excavation as required to maintain dry working conditions.
- Caisson/piers are to be poured within 24 hours after completion of drilling operation. Shoring requirements shall be determined by contractor. Contractor shall provide fall protection and safety barriers at and near the drilled hole as required by OSHA and the Authority Having Jurisdiction.
- The Contractor shall be responsible for all shoring, bracing, etc. necessary to support cut and/or fill banks, and existing structures during excavation, and the forming and placement of concrete.
- Bottom of caissons/piers shall be thoroughly cleaned prior to placement of concrete.

### STRUCTURAL STEEL AND WELDING

- All structural steel construction shall conform to AISC 360-16 and AISC 341-16.
  - Fabrication of all structural steel shall be done in the shop of an approved fabricator. Inspection and approval for fabricator's shops used for fabrication of structural load bearing members, components, materials or assemblies shall conform to CBC Section 1704A.2.5.
- All structural steel shall conform to the following specifications:
  - Angles, channels, plates, bars, rounds, and other miscellaneous shapes: Shall conform to ASTM A36 and shall have a minimum yield stress ( $F_y$ ) of 36 ksi.
  - Wide-flange shapes: Shall conform to ASTM A992 and shall have a minimum yield stress ( $F_y$ ) of 50 ksi.
  - Structural tubes: Shall be ASTM A500, Grade C, and shall have a min. yield stress ( $F_y$ ) of 50ksi.
- All structural steel fasteners shall conform to the following specifications:
  - Bolts shall conform to ASTM A307
  - Anchor Bolts shall conform to ASTM F1554, Grade as noted in drawings
  - Carbon steel nuts shall conform to ASTM A563
  - Stainless steel nuts shall conform to ASTM F594
  - Washers shall conform to ASTM F436
- Special Inspection shall be provided for all structural steel and welding, in accordance with CBC Chapter 17A.
- All structural steel shall be fabricated, erected and welded in accordance with AISC Specifications for Structural Steel Buildings (AISC 360-16) and Code of Standard Practice for Steel Buildings and Bridges (AISC 303-16).
- All welding shall be done by qualified and certified welders.
- Shop drawings for the fabrication of any structural steel shall be approved by the Contractor and submitted to Project Specific Architect or Project Specific Structural Engineer of Record for their review, prior to fabrication.
- No holes other than those specifically detailed shall be allowed through structural steel members. Burning of holes is not permitted.
- All welding shall conform to "AWS D1.1" specifications for welding. (E-70XX Electrodes).
- Where fillet weld size is not indicated, use "AWS" minimum size based on the thickness of the thinner part being welded, as specified in AISC Specifications for Structural Steel Buildings (AISC 360-10), Section J2.2.
- All butt welds to be complete joint penetration, unless specifically noted otherwise.
- Welder qualification requirements, welding procedure and welding electrodes for all structural steel (except structural sheet steel, see steel decking) shall conform to CBC Sections 1705A.2.1 and 2204A.1.
- Provide 3" minimum concrete cover around all structural steel below grade.
- Structural steel embedded into concrete shall be uncoated.
- Structural steel shall be hot-dip galvanized (minimum ASTM A123 or A153 Class D) or painted with zinc-rich primer, undercoat, and finish coat; or equivalent paint system.
- All exposed steel fasteners, including cast-in-place anchor bolts/rods, shall be stainless steel (Type 304 minimum), hot-dip galvanized (ASTM A153, Class D minimum or ASTM F2239), or protected with corrosion preventive coating that demonstrated no more than 2% of red rust in minimum 1,000 hours of exposure in salt spray test per ASTM B117. Zinc plated fasteners do not comply with this requirement.

## ABBREVIATIONS

A.B.	Anchor Bolt	HORIZ.	Horizontal
ABV.	Above	HSS	Hollow Steel Section
ACI	American Concrete Institute	HT.	Height
ADJ.	Adjacent	ICC	International Building Code
AHJ	Division of the State Architect	ICC	International Code Council
ASC	American Institute of Steel Construction	ID	Inside Diameter
AOR	Architect of Record	IN.	Inch, Inches
APPROX.	Approximately	INT.	Interior
ASCE	American Society of Civil Engineers	ksi	Kips per Square Inch
ARCH.	Architect, Architecture	LL	Live Load
ASTM	American Society of Testing and Materials	MAX.	Maximum
ATR	All Thread Rod	MB	Machine Bolt
AWG	American Welding Society	MFR	Manufactured, Manufacturer
B.O.	Bottom of _____	MIN.	Minimum
BOT.	Bottom	MPH	Miles per Hour
b/t	Between	N/R	Not Required
CAC	California Administrative Code	N.T.S.	Not to Scale
CBC	California Building Code	o.c.	On Center
CIP	Cast-in-place	o/	Over
CJP	Complete Joint Penetration	OD	Outside Diameter
CL	Centerline	PEN.	Penetration
CLR.	Clear	PL	Plate
COL.	Column	PIP	Partial Joint Penetration
CONC.	Concrete	psf	Pounds per Square Foot
CONJ.	Connection	PSF	Pounds per Square Foot
CONST.	Construction	REBAR	Reinforcing Bar
CONT.	Continue, Continuous	REINF.	Reinforcement
Ø	Diameter	REQD	Required
DBL	Double	S.F.	Square Feet
DET.	Detail	SHT	Sheet
DL	Dead Load	SIM.	Similar
DSA	Division of State Architect	SMS	Sheet Metal Screw
DWGS.	Drawings	SQ.	Square
EA	Each	STAGCD	Staggered
E.F.	Each Face	STD.	Standard
ELEC.	Electric, Electrical	STL	Steel
ELEV.	Elevation	SEOR	Structural Engineer of Record
EMBED.	Embedded, Embedment	T.O.	Top of _____
EOR	Engineer of Record	T&B	Top and bottom
EJ	Equal	THRD	Threaded
EQUIP.	Equipment	TYP.	Typical
E.S.	Each Side	U.N.O.	Unless Noted Otherwise
E.W.	Each Way	VERT.	Vertical
EXT.	Exterior	VIF	Verify in Field
FAB.	Fabricated	w/	With
FDN.	Foundation	w/c	Water-Cement Ratio
F.G.	Finish Grade	WSS	Welded Steel Stud
F.O.	Face of _____	WT.	Weight
FRMG.	Framing		
FT.	Foot, Feet		
FTG.	Footing		
GA.	Gauge		
GALV.	Galvanized		
GEOR.	Geotechnical Engineer of Record		

### POST INSTALLED ANCHOR & TESTING

- All post-installed anchors are to be tension tested with the exception that torque testing is allowed if the anchors are specifically designed as torque controlled

Application	Quantity
Non-structural (Equipment Anchorage, etc.)	50%
Structural	100%

- Test quantity of post-installed anchors as noted below:
 

Application	Quantity
Non-structural (Equipment Anchorage, etc.)	50%
Structural	100%
- Apply proof test loads to anchors without removing the nut if possible. If not, remove nut and install a threaded coupler to the same tightness of the original nut using a torque wrench and apply load.
- All tests shall be performed in the presence of the inspector.
- Reaction loads from test fixtures may be applied close to the anchor being tested, provided the anchor is not restrained from withdrawing or restricted from a concrete shear cone type failure mechanism.
- Test equipment is to be calibrated by an approved testing laboratory in accordance with standard recognized procedures.
- The following criteria apply for the acceptance of installed anchors:
  - Hydraulic ram method: anchors tested with a hydraulic jack or spring loaded devices shall maintain the test load for a minimum of 15 seconds and shall exhibit no discernible movement during the tension test, e.g. as evidenced by loosening of the washer under the nut.
  - Torque wrench method: anchors tested with a calibrated torque wrench must attain the manufacturer recommended torque within 1/2 turn of the nut.
    - Wedge or sleeve type: one-quarter turn of the nut from 3/8" sleeve anchor only.
    - Threaded type: one-quarter turn of the screw after initial seating of the screw head.
- If any anchor fails testing, test all anchors of the same type not previously tested until twenty consecutive anchors pass, then resume the initial test frequency. If the anchors are used for the support and bracing of non-structural components (pipe, duct or conduit), the twenty shall be only those anchors installed by the same trade.
- Test loads per ICC ESR, IAPMO, OR UES report
- When installing drilled-in anchors and/or powder driven pins in existing non-prestressed reinforced concrete, use care and caution to avoid cutting or damaging the existing reinforcing bars. When installing them into existing prestressed concrete (pre- or post-tensioned) locate the prestressed tendons by using a non-destructive method prior to installation. Exercise extreme care and caution to avoid cutting or damaging the tendons during installation. Maintain a minimum clearance of one inch between the reinforcement and the drilled-in anchor and/or pin.

### ANCHOR TORQUE TEST VALUES

Anchor Diameter	CONCRETE		MASONRY	
	HILTI KB TZ 2	SIMPSON STRONG BOLT Z	HILTI KB TZ 2	SIMPSON STRONG BOLT Z
ESR-4266	ESR-3037	ESR-4561	ER-240	
3/8"	30 ft-lb	30 ft-lb	15 ft-lb	20 ft-lb
1/2"	50 ft-lb	60 ft-lb	25 ft-lb	35 ft-lb
5/8"	40 ft-lb	90 ft-lb	30 ft-lb	55 ft-lb
3/4"	110 ft-lb	150 ft-lb	50 ft-lb	100 ft-lb

If the manufacturer's recommended installation torque is less than the test torque noted in the table, the manufacturer's recommended installation torque should be used in lieu of the tabulated values.

See manufacturer's ESR report for Maximum Impact Wrench Torque Rating.

APPLICATION #  
02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC:  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

SSG  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA R. RANNEY  
No. 5405  
STATE OF CALIFORNIA  
08.09.2023

PC SEOR SEAL

THESE DRAWINGS, NOTES AND DETAILS ARE INSTRUMENTS OF SERVICE AND ARE THE PROPERTY OF SSG STRUCTURAL ENGINEERS, LLP. ALL DRAWINGS, INFORMATION, SPECIFICATIONS, FEAS, RENDERS AND AMENDMENTS REPRESENTED WITHIN THESE DOCUMENTS SHALL REMAIN THE PROPERTY OF THE ENGINEER. NO PART THEREOF SHALL BE COPIED, REPRODUCED OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED AND DEVELOPED WITHOUT THE EXPRESS WRITTEN CONSENT OF THE ENGINEER, CONTRACTOR OR ARCHITECT.  
THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

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APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122377 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2024

DSA STAMP

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA R. RANNEY  
No. 52366  
3-31-24  
STATE OF CALIFORNIA  
08.09.2023

STRUCTURAL  
NOTES &  
SPECIAL  
INSPECTIONS

SHEET INFORMATION

DATE 08.09.2023

DRAWN JMK

CHECKED MEP

SSG JOB # S23109

SHEET SBO.2



**ONE COLUMN ASSEMBLY - CASE 1**

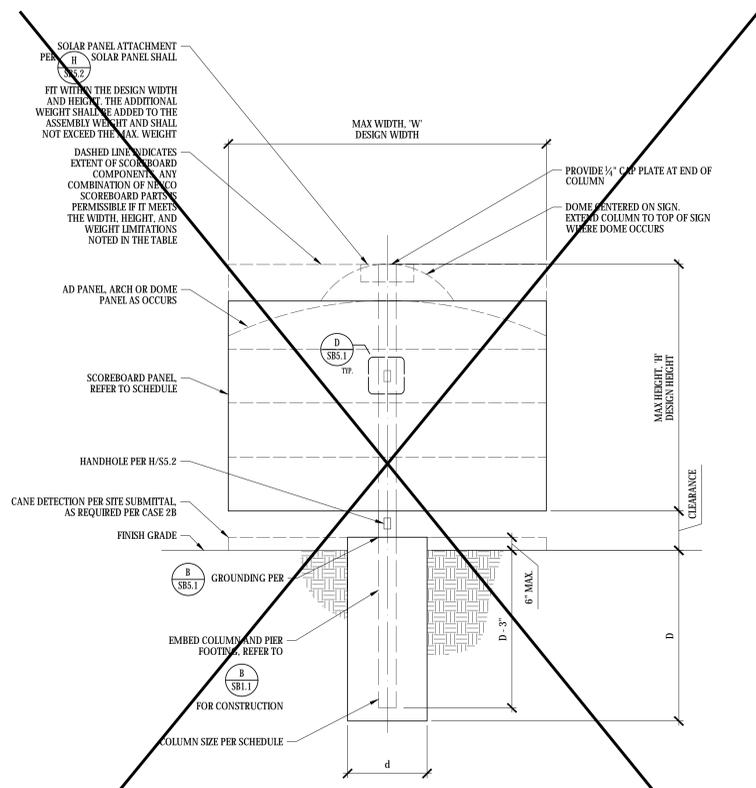
ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	9'-0"	N/R	N/R

NOTES:  
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06

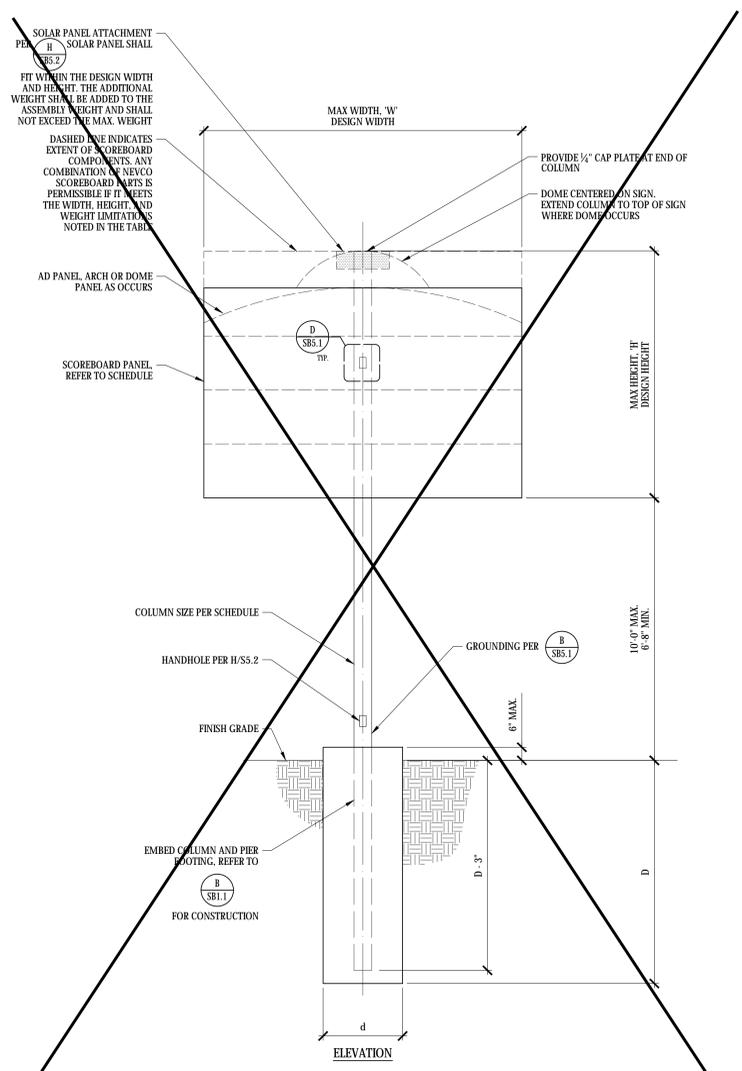
**ONE COLUMN ASSEMBLY - CASE 2A<sup>(2)</sup> AND CASE 2B<sup>(2)(3)</sup>**

ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF.
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	5'-3"	N/R	N/R
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{5}{8}$	30"Ø	6'-6"	N/R	N/R

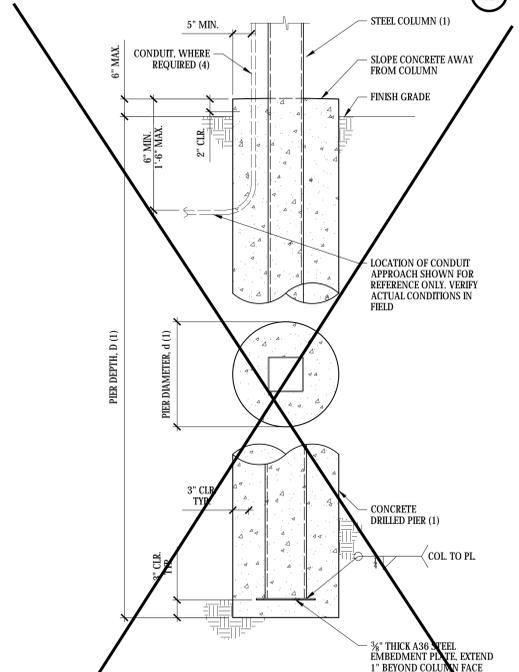
NOTES:  
1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06  
2. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
3. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD



**CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"**  
**CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6'-8"**



**CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE**



**CONCRETE DRILLED PIER**

**ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION**

N.T.S.

APPLICATION # 02-122088  
IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC:  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

**SSG**  
structural engineers  
REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. FAYON  
No. 5485  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023  
PC SIGNER REAL

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Phone: (618) 664-0960  
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APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122088 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT  
CODE: 2022  
A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
CALYD R. RANOLD  
No. 52386  
EXPIRES 3-31-24  
STATE OF CALIFORNIA

**MARQUEE CAISSON - EMBEDDED**

SHEET INFORMATION

DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	<b>SB1.1</b>

**ONE COLUMN ASSEMBLY - CASE 1**

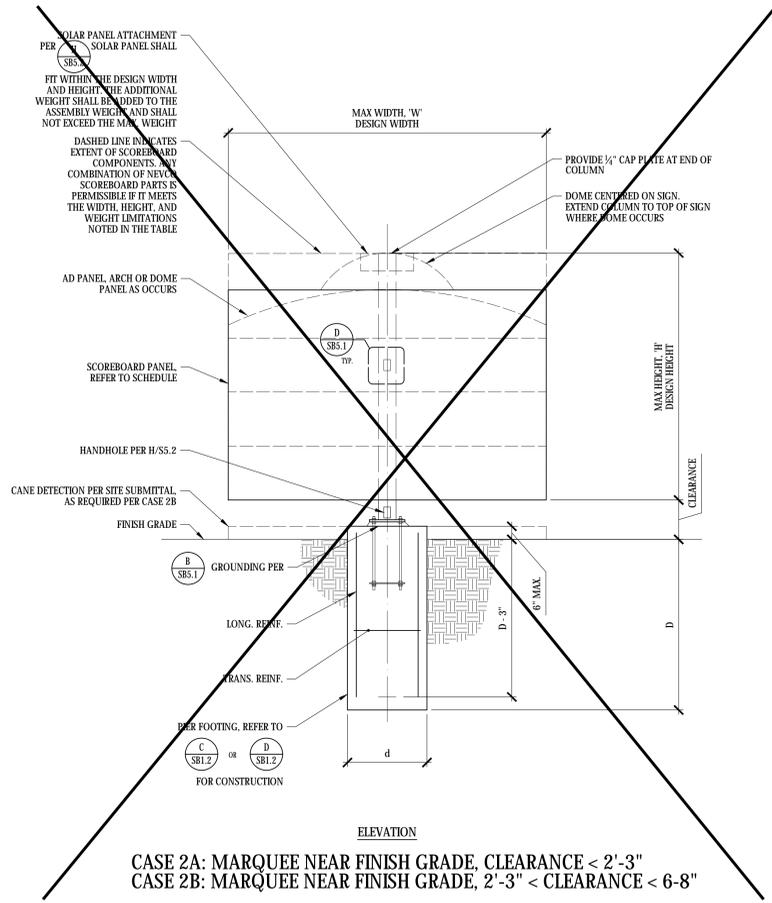
ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (2)					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30" $\varnothing$	6'-6"	8 - #6	#4 @ 4 $\frac{1}{2}$ " o.c.
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30" $\varnothing$	9'-0"	8 - #6	#4 @ 4 $\frac{1}{2}$ " o.c.

NOTES: (#)  
 1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.  
 2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.

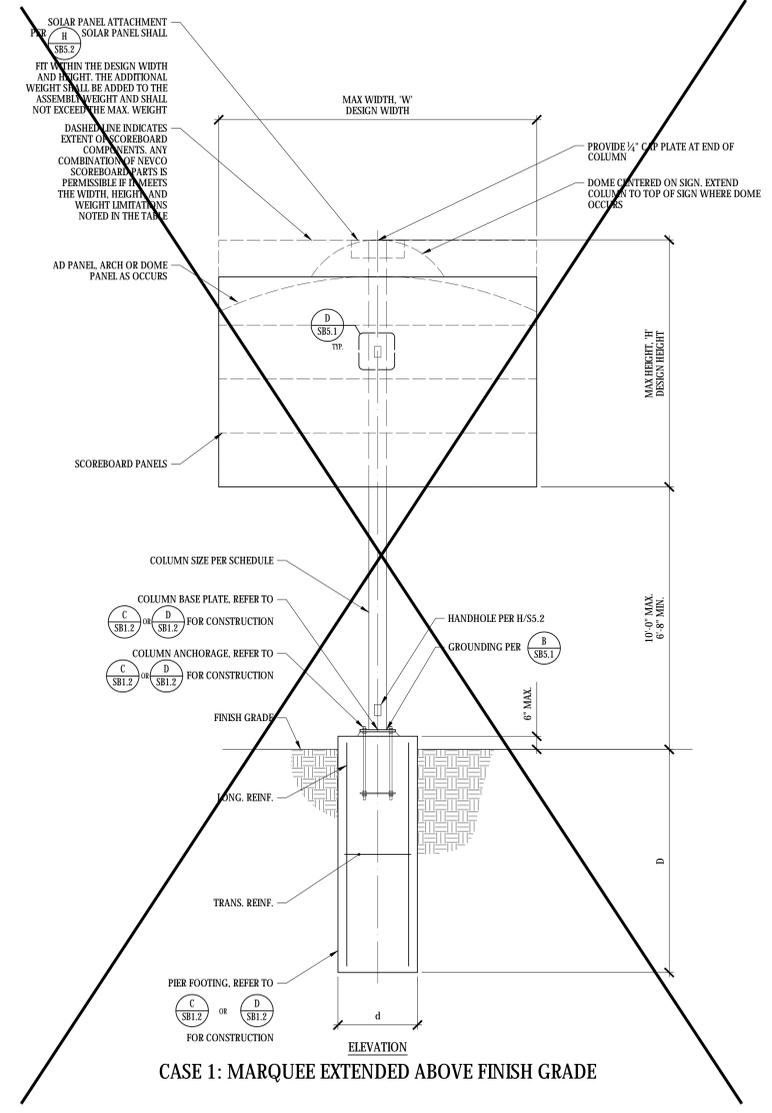
**ONE COLUMN ASSEMBLY - CASE 2A(3) AND CASE 2B(3)(4)**

ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (2)					
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x $\frac{3}{8}$	30" $\varnothing$	5'-3"	8 - #6	#4 @ 4 $\frac{1}{2}$ " o.c.
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x $\frac{3}{8}$	30" $\varnothing$	6'-6"	8 - #6	#4 @ 4 $\frac{1}{2}$ " o.c.

NOTES: (#)  
 1. CONTRACTOR OPTION TO PROVIDE TIE OR SPIRAL REINFORCING. SEE C/SB1.2 FOR TIE OPTION, SEE D/SB1.2 FOR SPIRAL OPTION.  
 2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION.  
 3. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
 4. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD.



**CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"**  
**CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6-8"**



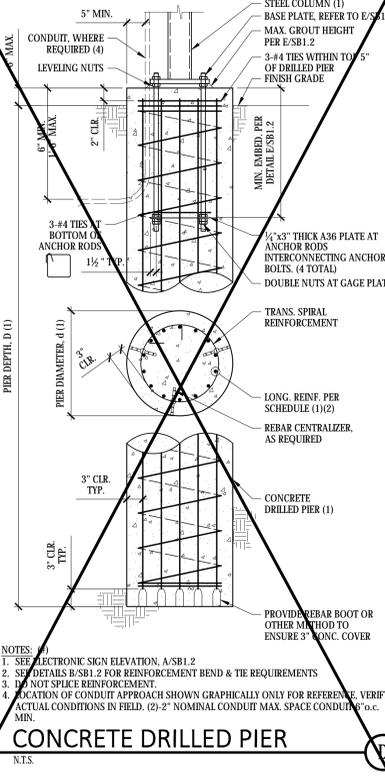
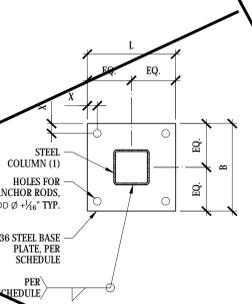
**CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE**

**ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION**

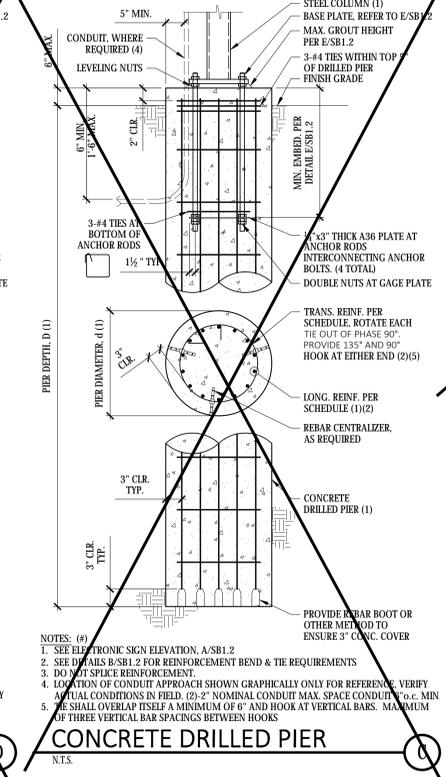
**BASE PLATE SCHEDULE**

COLUMN SIZE	BASE PLATE			WELD	QUANTITY & DIAMETER	ANCHOR ROD	
	THICKNESS, t	WIDTH, B	LENGTH, L			DEPTH, X	GROUT HEIGHT
HSS8x8x $\frac{3}{8}$	1"	16"	16"	3" FILLET	4-1 $\frac{1}{2}$ " $\varnothing$	E1554	2 $\frac{1}{2}$ " 1 $\frac{1}{2}$ "

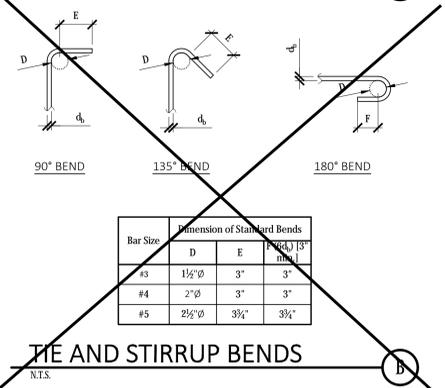
NOTES: (#)  
 1. SEE SCOREBOARD ELEVATION, A/SB1.2  
**BASE PLATE**  
 N.T.S.



**CONCRETE DRILLED PIER**  
 N.T.S.



**CONCRETE DRILLED PIER**  
 N.T.S.



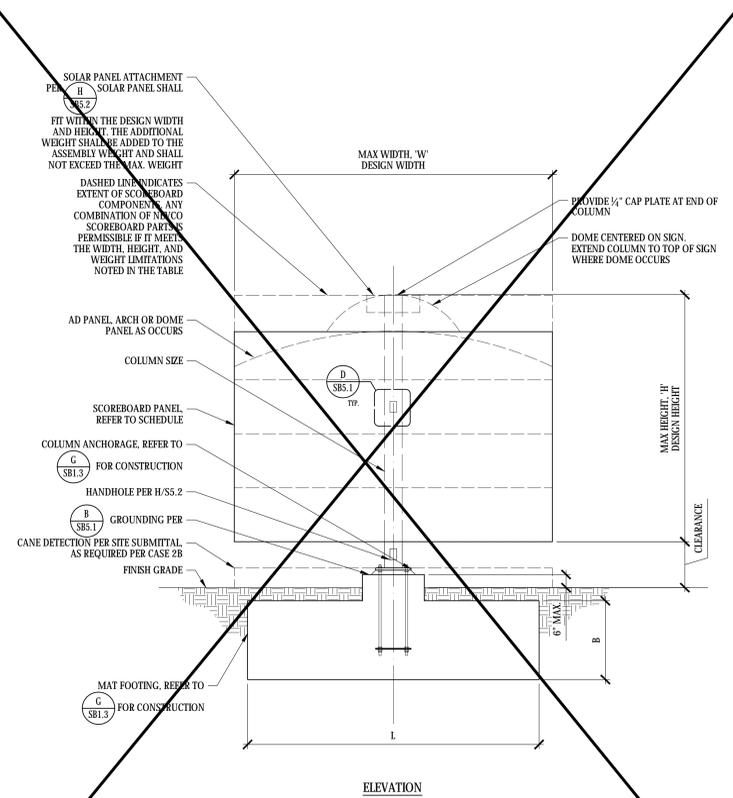
**TIE AND STIRRUP BENDS**  
 N.T.S.

NOTES: (#)  
 1. SEE ELECTRONIC SIGN ELEVATION, A/SB1.2  
 2. SEE DETAILS B/SB1.2 FOR REINFORCEMENT BEND & TIE REQUIREMENTS  
 3. DO NOT SPLICE REINFORCEMENT.  
 4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT o.c. MIN.  
 5. WE SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

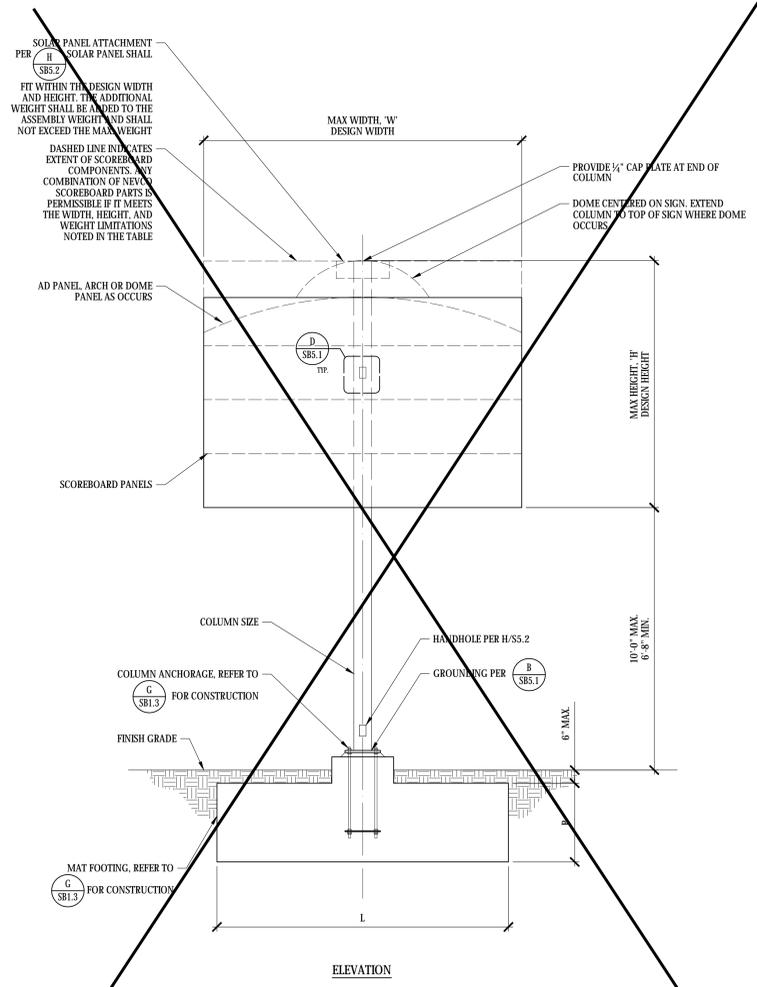
ONE COLUMN ASSEMBLY - CASE 1							
ASSEMBLY CRITERIA				MAT FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x6	6'-0"	3'-0"	5'-0"
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x6	7'-0"	3'-0"	7'-0"

ONE COLUMN ASSEMBLY - CASE 2A(2) AND CASE 2B(2)(3)							
ASSEMBLY CRITERIA				MAT FOOTING CRITERIA			
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SIZE	WIDTH, A	DEPTH, B	LENGTH, L
8'-0"		570 lbs.	≤ 4'-0"	HSS8x8x6	4'-6"	2'-6"	4'-6"
10'-0"		1,535 lbs.	≤ 8'-0"	HSS8x8x6	6'-0"	2'-6"	6'-0"

NOTES:  
 1. CASE 2 MAY NOT BE INSTALLED IN PATH OF TRAVEL. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
 2. CASE 2A AND 2B MAY NOT BE INSTALLED IN CIRCULATION PATH. DESIGN PROFESSIONAL OF RECORD SHALL INDICATE ON SITE PLAN.  
 3. CANE DETECTION DESIGN SHALL BE PROVIDED AS PART OF SITE SUBMITTAL PACKAGE BY DESIGN PROFESSIONAL OF RECORD

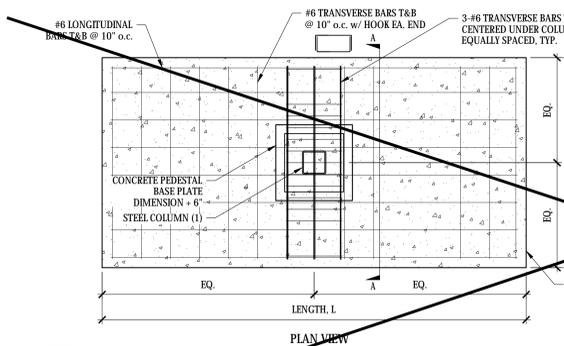


CASE 2A: MARQUEE NEAR FINISH GRADE, CLEARANCE < 2'-3"  
 CASE 2B: MARQUEE NEAR FINISH GRADE, 2'-3" < CLEARANCE < 6'-8"

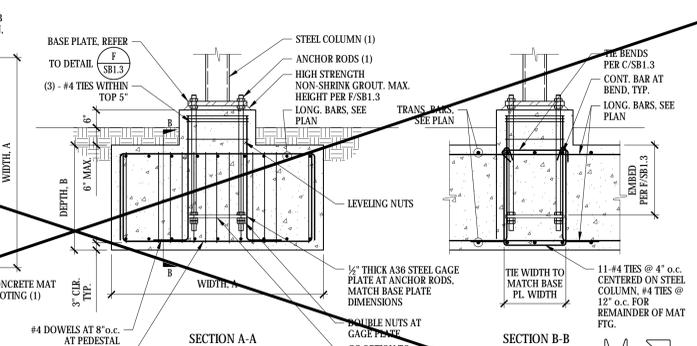


CASE 1: MARQUEE EXTENDED ABOVE FINISH GRADE

ONE COLUMN MARQUEE/SCOREBOARD INSTALLATION



MAT FOOTING CONSTRUCTION AND ANCHORAGE

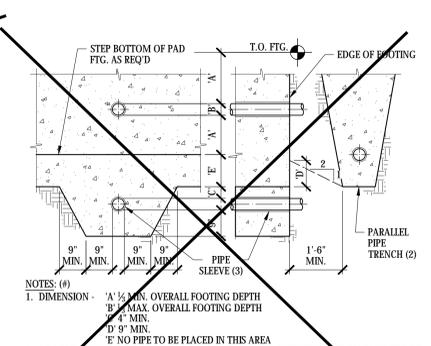


SECTION A-A  
 SECTION B-B

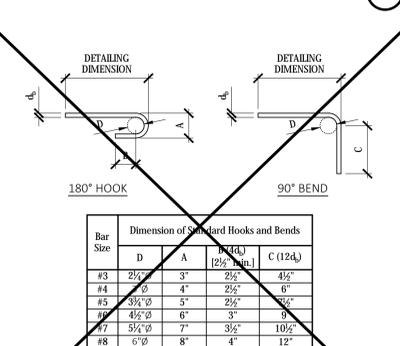
COLUMN SIZE	BASE PLATE			ANCHOR ROD				
	THICKNESS, t	WIDTH, B	LENGTH, L	QUANTITY & DIAMETER	GRADE	WELD	GROUT HEIGHT	EMBED.
HSS8x8x6	1"	16"	16"	3/4" RILEY	4-1/2" A36	F1554	2 1/2"	1 1/2"



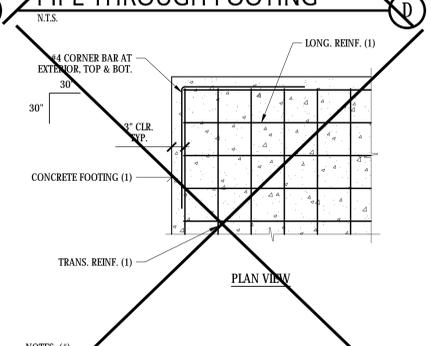
BASE PLATE



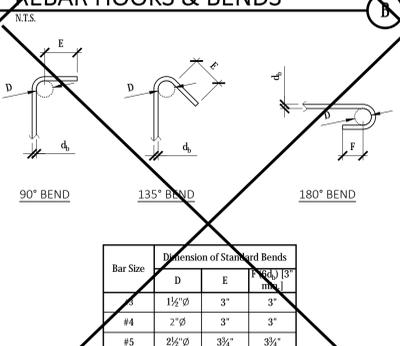
PIPE THROUGH FOOTING



REBAR HOOKS & BENDS



FOOTING CORNER



TIE AND STIRRUP BENDS

APPLICATION # 02-122088  
 IDENTIFICATION STAMP  
 DIV. OF THE STATE ARCHITECT  
 APP: 02-122088 INC.  
 REVIEWED FOR  
 SS  FLS  ACS   
 DATE: 04/02/2024

**SSG**  
 structural engineers  
 REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL E. PARSONS  
 No. 5485  
 STATE OF CALIFORNIA  
 DATE SIGNED: 08.09.2023

**NEVCO**  
 301 East Harris Avenue, Greenville, Illinois 62246  
 Phone: (618) 664-0960  
 www.nevco.com

APPROVED  
 DIV. OF THE STATE ARCHITECT  
 APP: 04-122088 PC  
 REVIEWED FOR  
 SS  FLS  ACS  CG   
 DATE: 09/20/2023

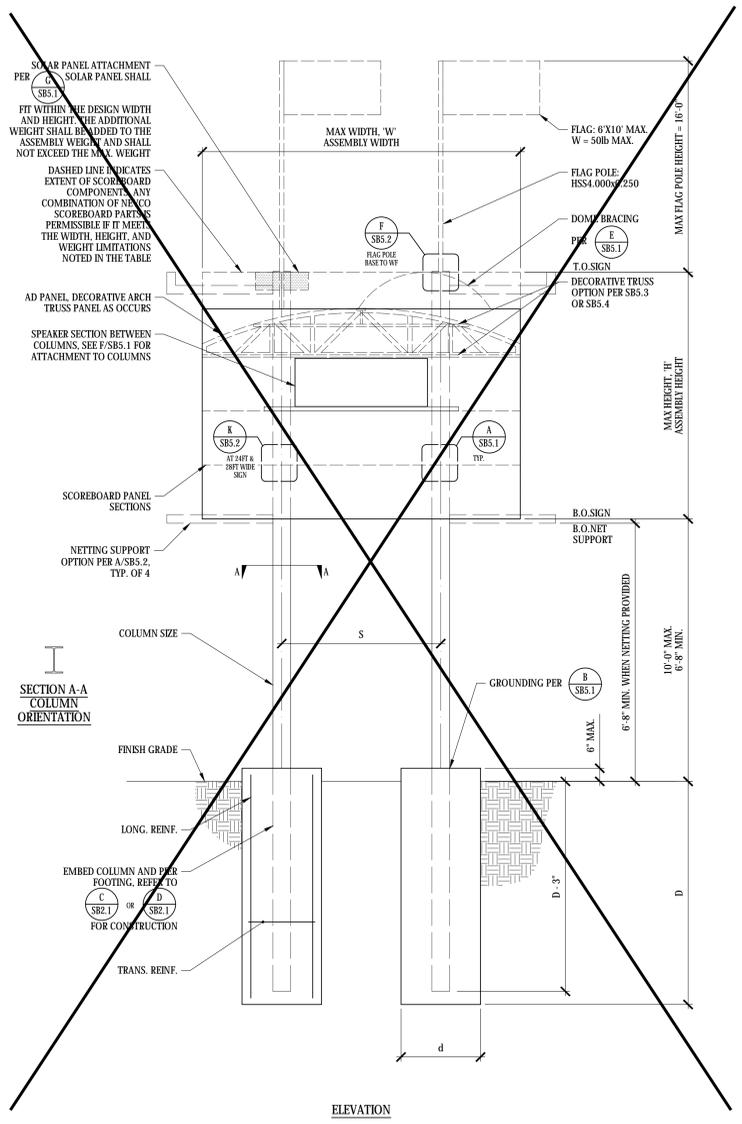
PRE-CHECK (PC) DOCUMENT  
 CODE: 2022  
 A separate project application for construction is required.

**REGISTERED PROFESSIONAL ENGINEER**  
 CALIFORNIA R. RANOLD  
 No. 52366  
 3-31-24  
 STATE OF CALIFORNIA

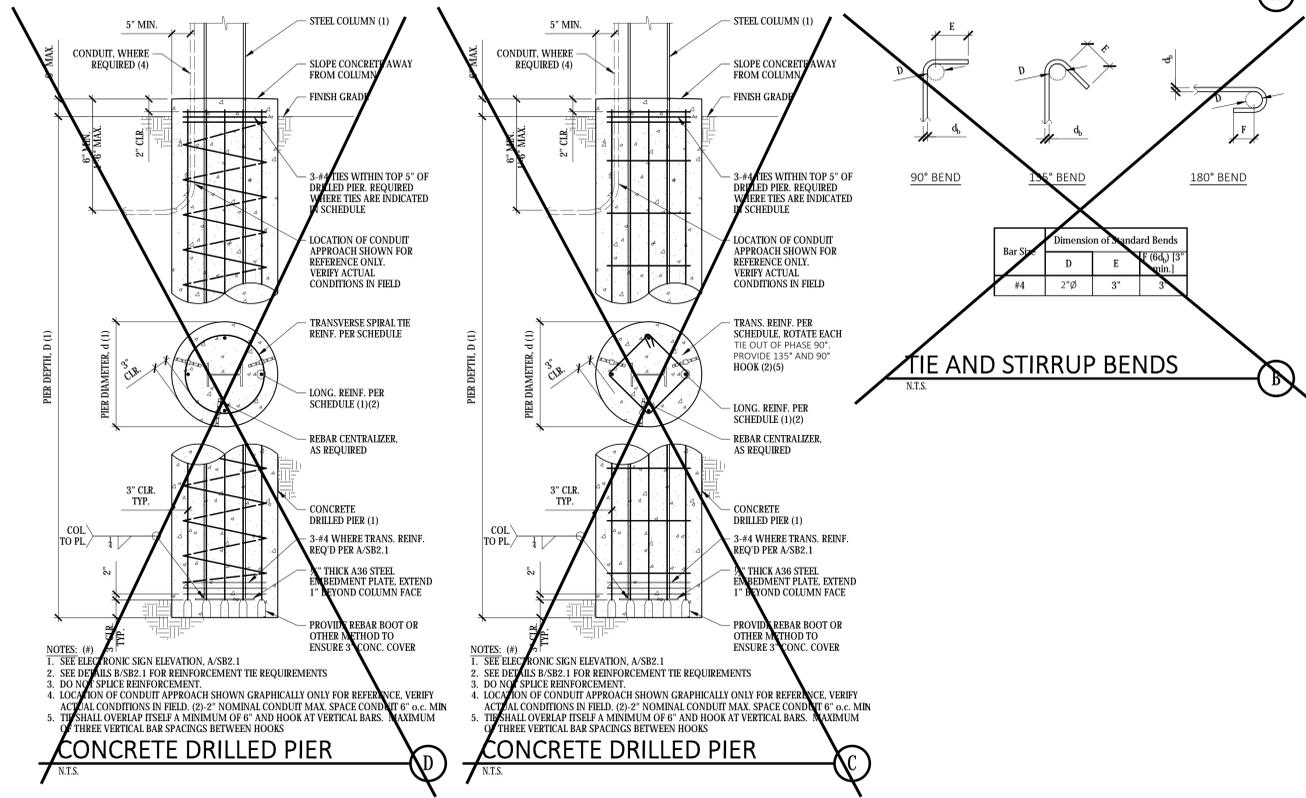
MARQUEE MAT FOOTING  
 SHEET INFORMATION  
 DATE: 08.09.2023  
 DRAWN: JMK  
 CHECKED: MEP  
 SSG JOB #: S23109  
 SHEET: SB1.3

TWO COLUMN ASSEMBLY										
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA					PIER FOOTING CRITERIA (3)			
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF. (#)	TRANS. REINF. (2)
8'-0"		770 lbs.	≤ 8'-0"	6'-0"	W8x24	W8x24	30"Ø	7'-6"	N/R	N/R
		1,160 lbs.	≤ 12'-0"	6'-0"	W10x33	W10x33	30"Ø	8'-9"	N/R	N/R
		1,540 lbs.	≤ 16'-0"	6'-0"	W12x40	W12x40	36"Ø	9'-0"	N/R	N/R
9'-0"		870 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	30"Ø	7'-9"	N/R	N/R
		1,300 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	30"Ø	7'-9"	N/R	N/R
		1,730 lbs.	≤ 16'-0"	8'-0"	W12x45	W12x40	36"Ø	9'-3"	4-#5	#4 @ 8" o.c.
10'-0"		960 lbs.	≤ 8'-0"	8'-0"	W8x24	W8x24	30"Ø	8'-0"	N/R	N/R
		1,440 lbs.	≤ 12'-0"	8'-0"	W10x33	W10x33	36"Ø	8'-6"	N/R	N/R
		1,920 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	36"Ø	9'-9"	4-#5	#4 @ 8" o.c.
12'-0"		2,400 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	9'-9"	4-#5	#4 @ 8" o.c.
		1,160 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	30"Ø	8'-6"	N/R	N/R
		1,730 lbs.	≤ 12'-0"	8'-0"	W14x43	W14x43	36"Ø	9'-3"	N/R	N/R
16'-0"		2,310 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	36"Ø	10'-3"	4-#5	#4 @ 8" o.c.
		2,880 lbs.	≤ 20'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-3"	4-#5	#4 @ 8" o.c.
		1,540 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	30"Ø	9'-6"	4-#5	#4 @ 8" o.c.
18'-0"		2,310 lbs.	≤ 12'-0"	8'-0"	W12x45	W12x40	36"Ø	10'-3"	4-#5	#4 @ 8" o.c.
		3,080 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	36"Ø	12'-0"	4-#5	#4 @ 8" o.c.
		3,840 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x77	48"Ø	12'-0"	4-#6	#4 @ 8" o.c.
24'-0"		1,730 lbs.	≤ 8'-0"	10'-0"	W12x35	W12x35	30"Ø	10'-0"	4-#5	#4 @ 8" o.c.
		2,600 lbs.	≤ 12'-0"	10'-0"	W14x43	W14x43	36"Ø	10'-9"	4-#5	#4 @ 8" o.c.
		3,460 lbs.	≤ 16'-0"	10'-0"	W14x61	W14x61	48"Ø	10'-9"	4-#6	#4 @ 8" o.c.
28'-0"		4,320 lbs.	≤ 20'-0"	10'-0"	W16x77	W16x77	48"Ø	12'-0"	4-#6	#4 @ 8" o.c.
		2,310 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	36"Ø	9'-9"	4-#5	#4 @ 8" o.c.
		3,460 lbs.	≤ 16'-0"	14'-0"	W14x61	W14x61	48"Ø	11'-9"	4-#5	#4 @ 8" o.c.
28'-0"		4,610 lbs.	≤ 20'-0"	14'-0"	W16x87	W16x87	48"Ø	13'-3"	4-#6	#4 @ 8" o.c.
		5,760 lbs.	≤ 24'-0"	14'-0"	W18x86	W18x86	48"Ø	14'-6"	4-#8	#4 @ 8" o.c.
		6,920 lbs.	≤ 28'-0"	14'-0"	W18x130	W18x119	48"Ø	14'-6"	4-#8	#4 @ 8" o.c.
28'-0"		8,070 lbs.	≤ 28'-0"	14'-0"	W18x158	W18x143	48"Ø	16'-0"	4-#8	#4 @ 6" o.c.
		2,600 lbs.	≤ 8'-0"	14'-0"	W14x43	W14x43	42"Ø	10'-0"	4-#6	#4 @ 6" o.c.
		4,040 lbs.	≤ 12'-0"	14'-0"	W14x61	W14x61	48"Ø	11'-3"	4-#6	#4 @ 6" o.c.
28'-0"		5,380 lbs.	≤ 16'-0"	14'-0"	W16x77	W16x77	48"Ø	12'-9"	4-#6	#4 @ 6" o.c.
		6,720 lbs.	≤ 20'-0"	14'-0"	W18x97	W18x97	48"Ø	13'-3"	4-#8	#4 @ 6" o.c.
		8,070 lbs.	≤ 24'-0"	14'-0"	W18x143	W18x143	48"Ø	15'-9"	4-#8	#4 @ 6" o.c.
28'-0"		9,410 lbs.	≤ 28'-0"	14'-0"	W18x175	W18x175	48"Ø	17'-0"	4-#8	#4 @ 6" o.c.

NOTES: (#)  
 1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06  
 2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB2.1 FOR THE OPTION. SEE D/SB2.1 FOR SPIRAL OPTION.  
 3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



**TWO COLUMN SCOREBOARD INSTALLATION**



NOTES: (#)  
 1. SEE ELECTRONIC SIGN ELEVATION, A/SB2.1  
 2. SEE DETAILS B/SB2.1 FOR REINFORCEMENT THE REQUIREMENTS  
 3. DO NOT SPLICE REINFORCEMENT.  
 4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN  
 5. TIES SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

APPLICATION # 02-122088

IDENTIFICATION STAMP  
 DIV. OF THE STATE ARCHITECT  
 APP: 02-122088 INC.  
 REVIEWED FOR  
 SS  FLS  ACS   
 DATE: 04/02/2024

**SSG**  
 structural engineers

REGISTERED PROFESSIONAL ENGINEER  
 CALIFORNIA R. RANSON  
 No. 5485  
 STATE OF CALIFORNIA  
 EXPIRES 08.09.2023

PC SEOR REAL

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

**NEVCO**  
 301 East Harris Avenue, Greenville, Illinois 62246  
 Phone: (618) 664-0960  
 www.nevco.com

APPROVED  
 DIV. OF THE STATE ARCHITECT  
 APP: 04-122088 PC  
 REVIEWED FOR  
 SS  FLS  ACS  CG   
 DATE: 09/20/2022

PRE-CHECK (PC) DOCUMENT  
 CODE: 2022

A separate project application for construction is required.

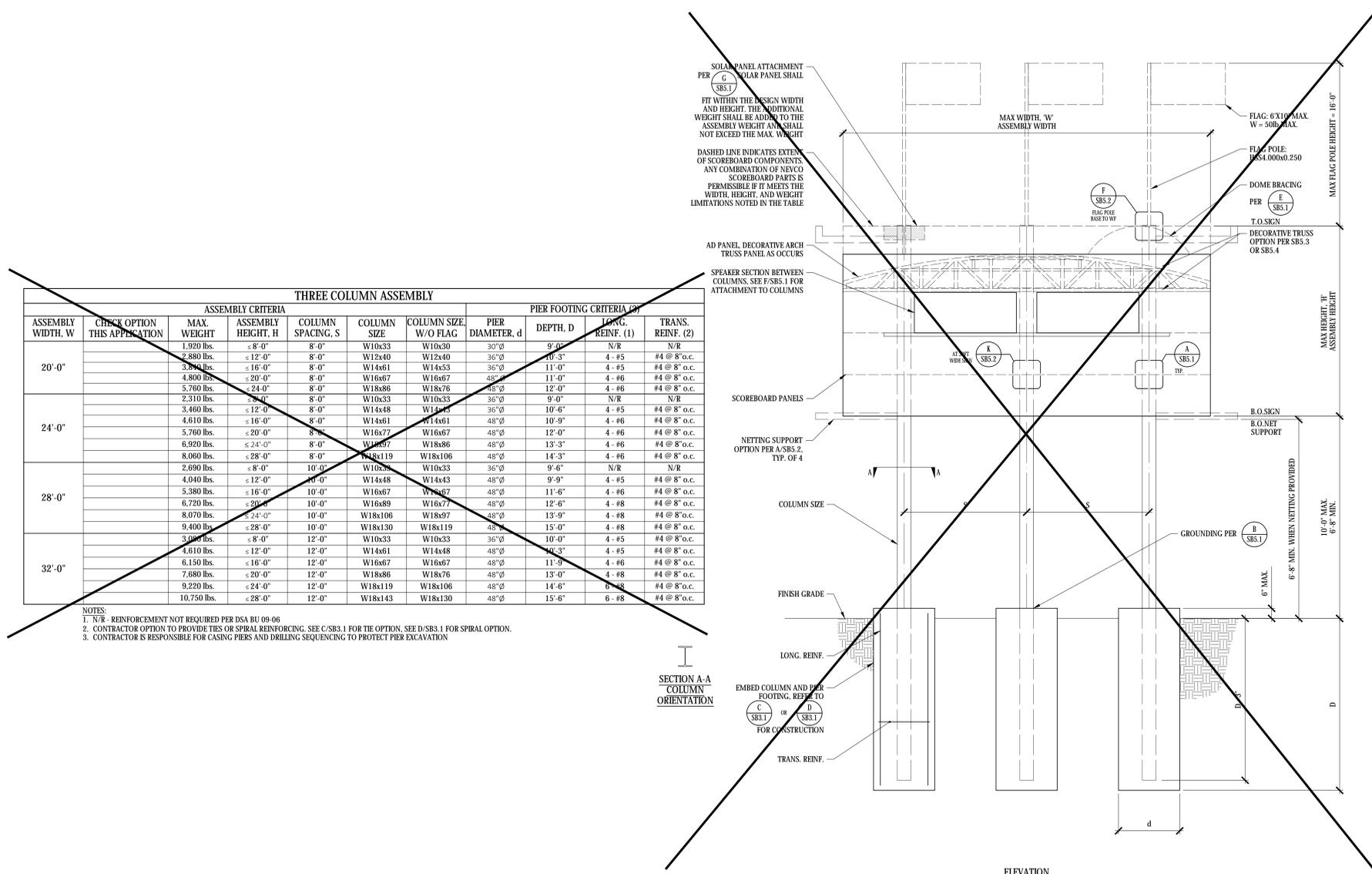
**TWO COLUMN CAISSON - EMBEDDED**

SHEET INFORMATION  
 DATE: 08.09.2023  
 DRAWN: JMK  
 CHECKED: MEP  
 SSG JOB #: S23109  
 SHEET: SB2.1





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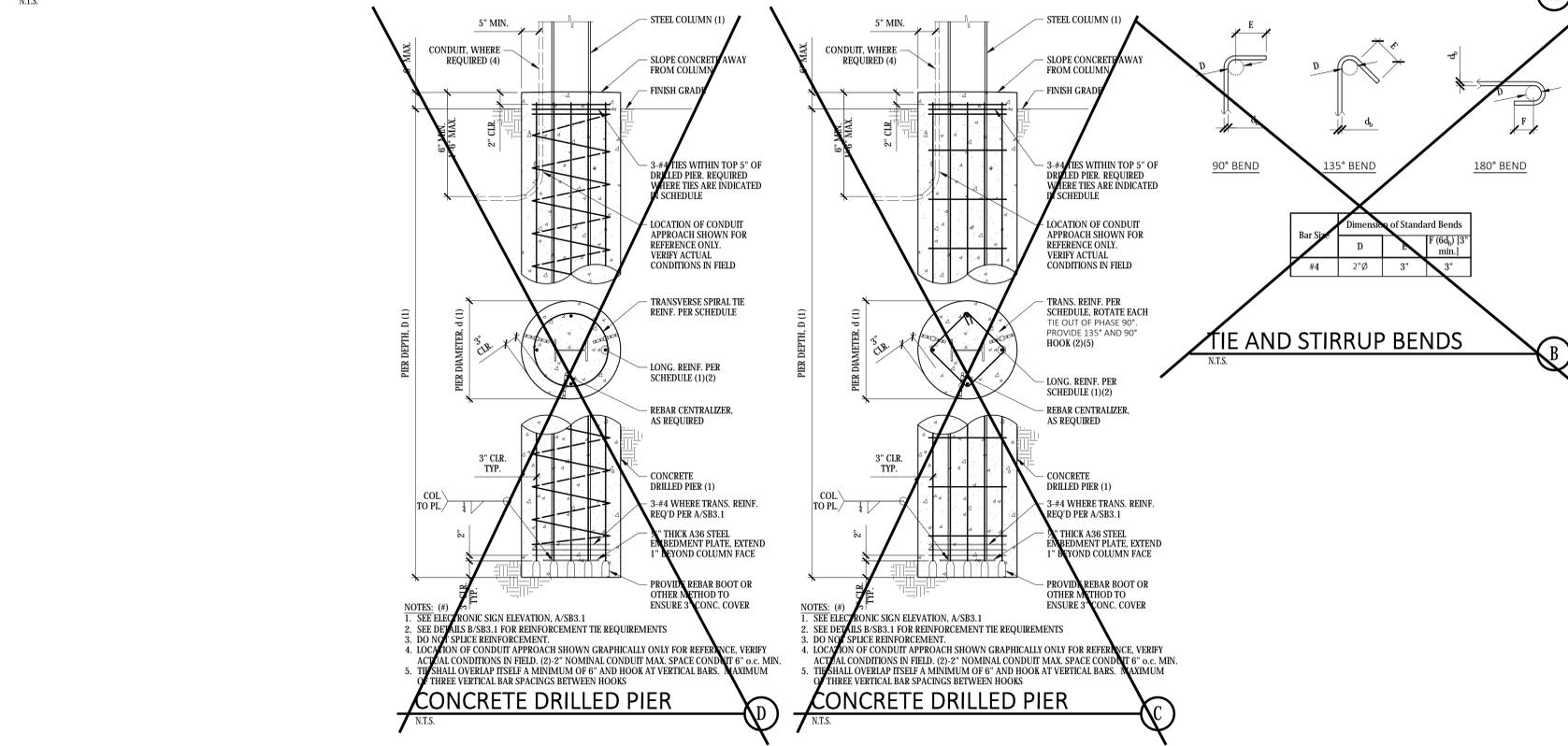


**THREE COLUMN ASSEMBLY**

ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA (3)					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF. (1)	TRANS. REINF. (2)
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	9'-0"	N/R	N/R
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	36"Ø	10'-3"	4-#5	#4 @ 8" o.c.
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	36"Ø	11'-0"	4-#5	#4 @ 8" o.c.
24'-0"		4,800 lbs.	≤ 20'-0"	8'-0"	W16x67	W16x67	48"Ø	11'-0"	4-#6	#4 @ 8" o.c.
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	4-#6	#4 @ 8" o.c.
		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-0"	N/R	N/R
28'-0"		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	36"Ø	10'-6"	4-#5	#4 @ 8" o.c.
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	48"Ø	10'-9"	4-#6	#4 @ 8" o.c.
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	48"Ø	12'-0"	4-#6	#4 @ 8" o.c.
32'-0"		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	48"Ø	13'-3"	4-#6	#4 @ 8" o.c.
		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	48"Ø	14'-3"	4-#6	#4 @ 8" o.c.
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R	N/R
	4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	36"Ø	9'-9"	4-#5	#4 @ 8" o.c.	
	5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-6"	4-#6	#4 @ 8" o.c.	
	6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	4-#8	#4 @ 8" o.c.	
	8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	4-#8	#4 @ 8" o.c.	
	9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	4-#8	#4 @ 8" o.c.	
	3,090 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	4-#5	#4 @ 8" o.c.	
	4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	11'-3"	4-#5	#4 @ 8" o.c.	
	6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	11'-9"	4-#6	#4 @ 8" o.c.	
	7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	4-#8	#4 @ 8" o.c.	
	9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	6-#6	#4 @ 8" o.c.	
	10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	48"Ø	15'-6"	6-#8	#4 @ 8" o.c.	

NOTES:  
 1. N/R - REINFORCEMENT NOT REQUIRED PER DSA BU 09-06  
 2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING, SEE C/SBS.1 FOR TIE OPTION, SEE D/SBS.1 FOR SPIRAL OPTION.  
 3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

**THREE COLUMN SCOREBOARD INSTALLATION**



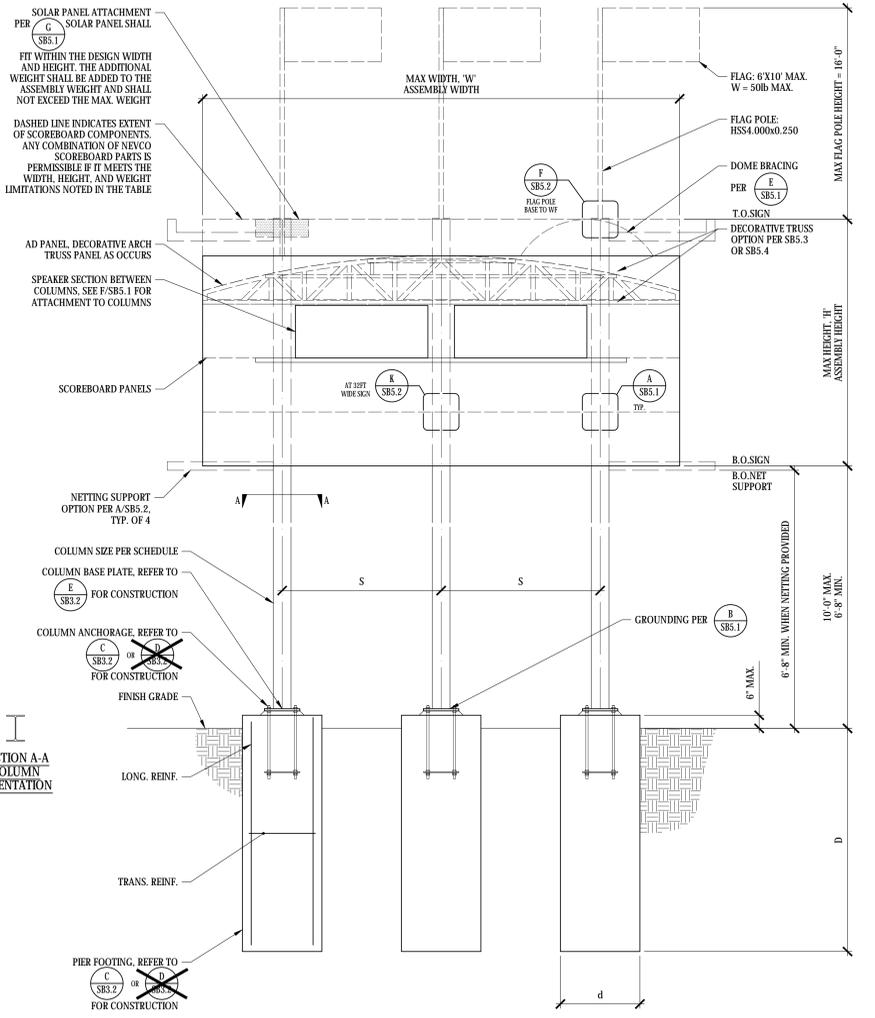
**CONCRETE DRILLED PIER**

**CONCRETE DRILLED PIER**

**TIE AND STIRRUP BENDS**

ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		COLUMN SPACING, S		COLUMN SIZE		PIER FOOTING CRITERIA (2)		BASE PLATE			ANCHOR RODS						
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED	
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	36"Ø	8'-6"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	3/16"	(4) - 1½"Ø	F1554 - GR.36	2½"	2"	48"
		2,880 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x40	36"Ø	10'-0"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	42"Ø	10'-0"	11-#8	#4 @ 6" o.c.	1½"	24"	24"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,800 lbs.	≤ 20'-0"	8'-0"	W18x67	W18x57	48"Ø	11'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	24"	3/16"	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
24'-0"		2,700 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	48"Ø	12'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		3,210 lbs.	≤ 8'-0"	8'-0"	W16x99	W16x92	36"Ø	10'-0"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	3/16"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
	X	3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	36"Ø	10'-6"	8-#8	#4 @ 6" o.c.	1½"	20"	20"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x51	42"Ø	10'-0"	10-#8	#4 @ 6" o.c.	1½"	24"	24"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
28'-0"		5,760 lbs.	≤ 30'-0"	8'-0"	W16x77	W16x67	48"Ø	10'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		6,880 lbs.	≤ 24'-0"	8'-0"	W18x87	W18x80	48"Ø	10'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		8,860 lbs.	≤ 30'-0"	8'-0"	W18x110	W18x106	48"Ø	11'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	3/16"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
32'-0"		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	48"Ø	9'-9"	8-#8	#4 @ 6" o.c.	1½"	24"	24"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-6"	14-#8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-0"	14-#8	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		8,070 lbs.	≤ 24'-0"	10'-0"	W18x106	W18x97	48"Ø	13'-9"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
32'-0"		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	48"Ø	15'-0"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.105	3"	2"	64"
		3,080 lbs.	≤ 12'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-0"	8-#6	#4 @ 4½" o.c.	1½"	20"	20"	3/16"	(4) - 1½"Ø	F1554 - GR.55	2½"	2"	48"
		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	10'-3"	8-#6	#4 @ 6" o.c.	1½"	24"	24"	3/16"	(6) - 1½"Ø	F1554 - GR.55	2½"	2"	64"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	11'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
32'-0"		7,680 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	48"Ø	13'-0"	14-#8	#4 @ 6" o.c.	1½"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	48"Ø	14'-6"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CJP	(6) - 1½"Ø	F1554 - GR.55	3"	2"	64"
32'-0"		10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	54"Ø	15'-6"	14-#9	#4 @ 6" o.c.	2½"	30"	36"	CJP	(6) - 2"Ø	F1554 - GR.55	4"	2"	64"

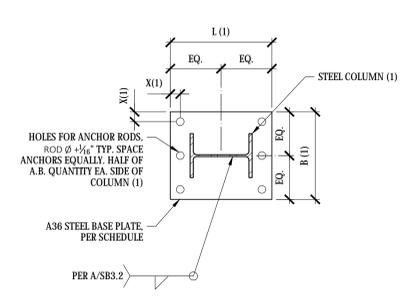
NOTES:  
1. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB3.2 FOR THE OPTION. SEE D/SB3.2 FOR SPIRAL OPTION.  
2. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION



ELEVATION

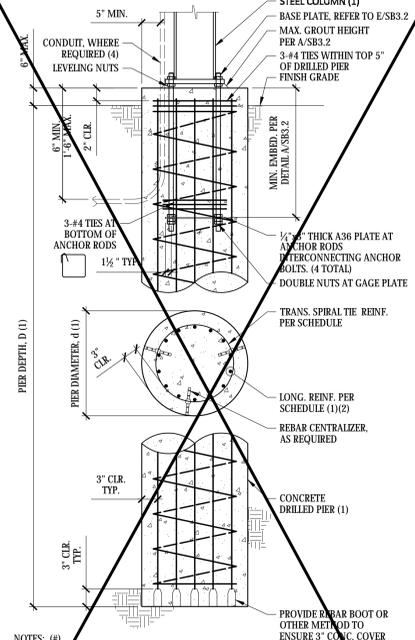
THREE COLUMN SCOREBOARD INSTALLATION

N.T.S.



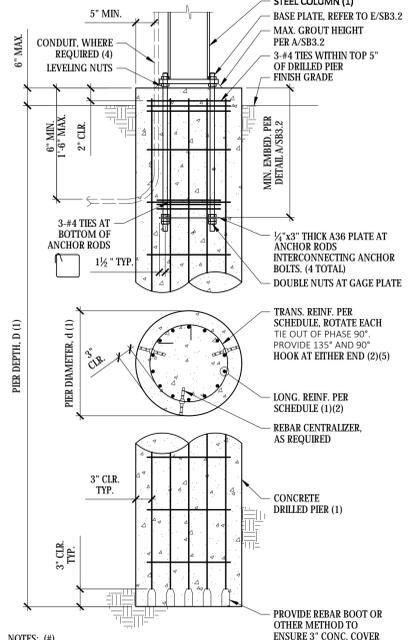
BASE PLATE

N.T.S.



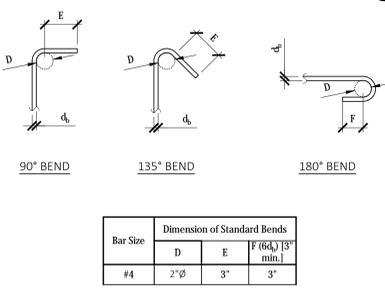
CONCRETE DRILLED PIER

N.T.S.



CONCRETE DRILLED PIER

N.T.S.



TIE AND STIRRUP BENDS

N.T.S.

NOTES: (4)  
1. SEE ELEVATION, A/SB3.2  
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPlice REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. TIES SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

NOTES: (4)  
1. SEE ELEVATION, A/SB3.2  
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPlice REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. TIES SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

NOTES: (4)  
1. SEE ELEVATION, A/SB3.2  
2. SEE DETAILS B/SB3.2 FOR REINFORCEMENT TIE REQUIREMENTS  
3. DO NOT SPlice REINFORCEMENT.  
4. LOCATION OF CONDUIT APPROACH SHOWN GRAPHICALLY ONLY FOR REFERENCE. VERIFY ACTUAL CONDITIONS IN FIELD. (2) 2" NOMINAL CONDUIT MAX. SPACE CONDUIT 6" o.c. MIN.  
5. TIES SHALL OVERLAP ITSELF A MINIMUM OF 6" AND HOOK AT VERTICAL BARS. MAXIMUM OF THREE VERTICAL BAR SPACINGS BETWEEN HOOKS

APPLICATION # 02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC.  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. RANOLD  
No. 5485  
STRUCTURAL  
STATE OF CALIFORNIA  
EXPIRES 08.09.2023

PC SEOR REAL

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

NEVCO

301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0980  
www.nevco.com

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122088 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2022

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL R. RANOLD  
No. 52366  
EXPIRES 3-31-24  
STRUCTURAL

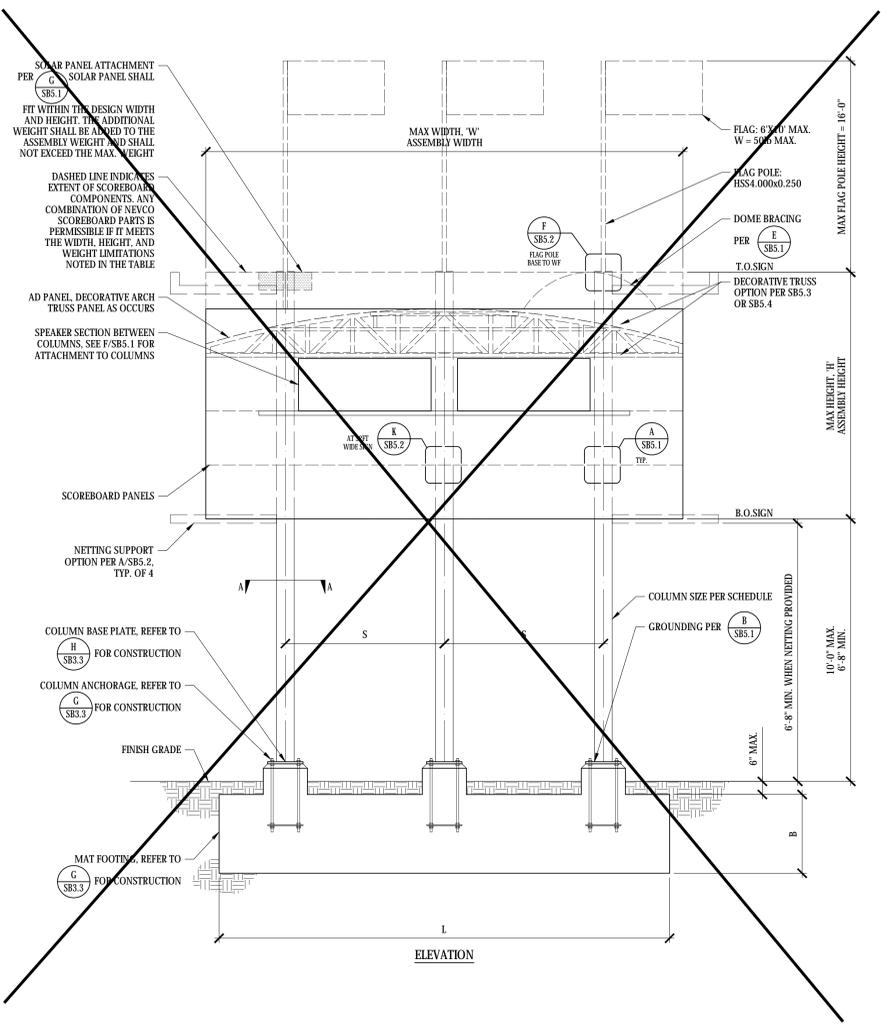
THREE COLUMN CAISSON - BOLTED

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB3.2

**THREE COLUMN ASSEMBLY**

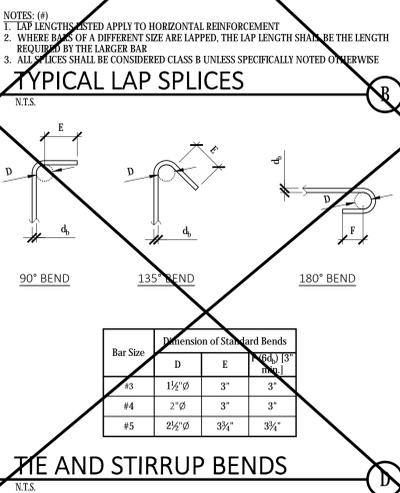
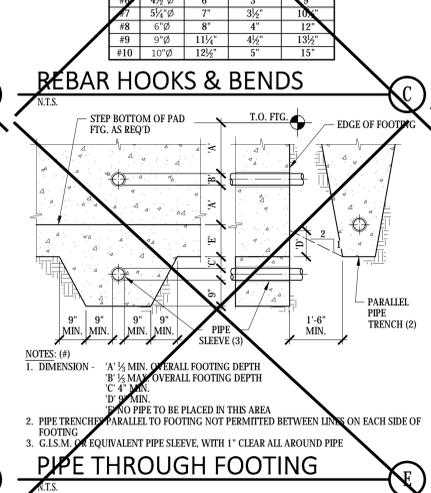
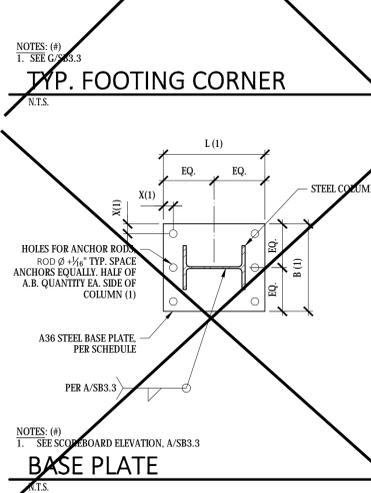
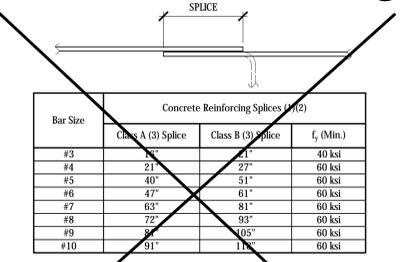
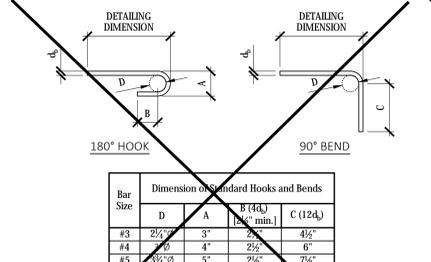
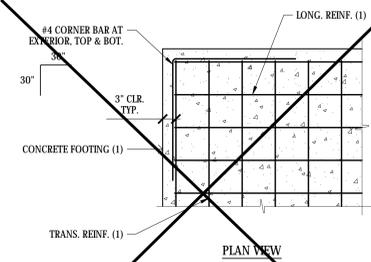
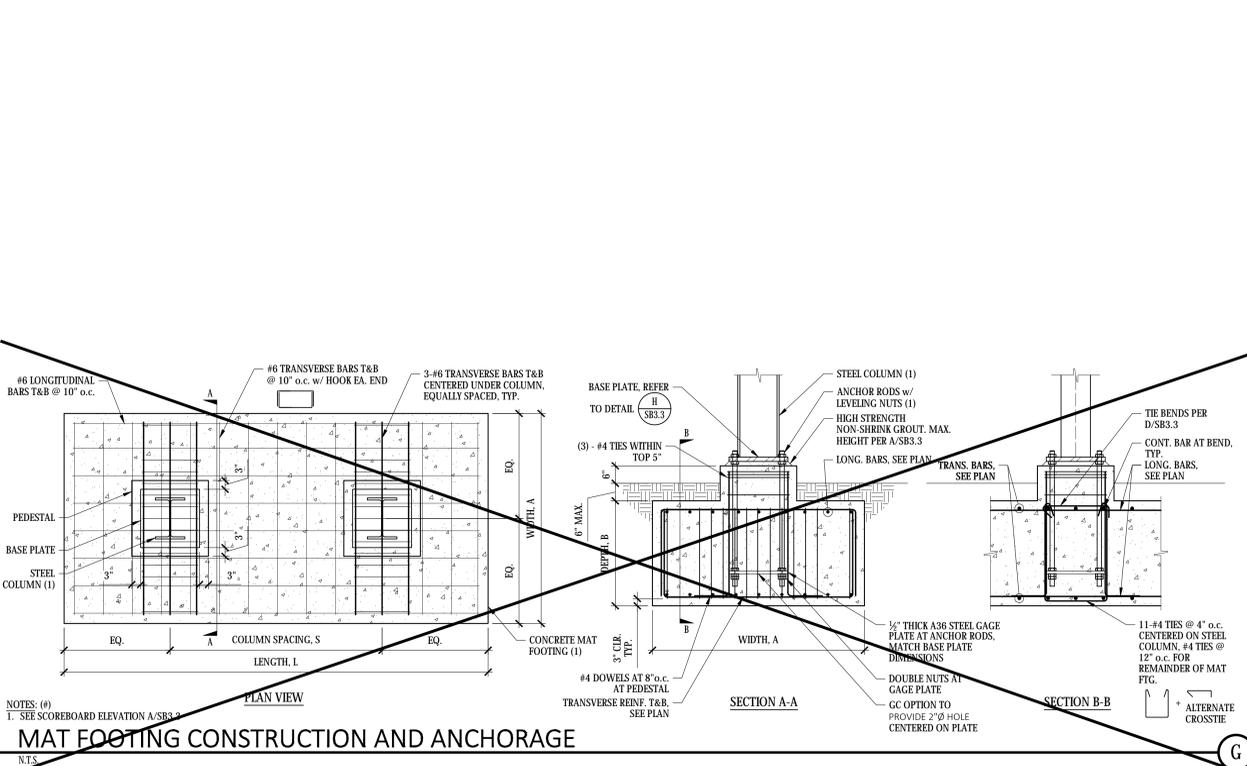
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		MAT FOOTING CRITERIA			BASE PLATE			ANCHOR RODS								
		MAX WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
20'-0"		1,920 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x30	8'-0"	3'-0"	20'-0"	1 1/2"	20"	20"	3/8"	(4) 1 1/2" ∅	F1554-GR.36	2 1/4"	2"	30"
		2,880 lbs.	≤ 12'-0"	8'-0"	W12x40	W12x40	9'-0"	3'-0"	23'-0"	1 1/2"	20"	20"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
		3,840 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x53	11'-0"	3'-0"	24'-0"	1 1/2"	24"	24"	3/8"	(8) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
24'-0"		4,800 lbs.	≤ 20'-0"	8'-0"	W16x86	W16x67	12'-0"	3'-0"	27'-0"	1 1/2"	24"	30"	3/8"	(4) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		5,760 lbs.	≤ 24'-0"	8'-0"	W18x86	W18x76	14'-0"	3'-0"	27'-0"	1 1/2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		2,310 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	23'-0"	1 1/2"	20"	20"	3/8"	(4) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
28'-0"		3,460 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	9'-0"	3'-0"	26'-0"	1 1/2"	20"	20"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	28'-0"	1 1/2"	24"	24"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
		5,760 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	12'-0"	3'-0"	30'-0"	1 1/2"	24"	30"	3/8"	(6) 1 1/2" ∅	F1554-GR.105	2 1/4"	2"	30"
32'-0"		6,920 lbs.	≤ 24'-0"	8'-0"	W18x97	W18x86	14'-0"	3'-0"	30'-0"	1 1/2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		8,060 lbs.	≤ 28'-0"	8'-0"	W18x119	W18x106	15'-0"	3'-0"	30'-0"	2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		2,690 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/2"	20"	20"	3/8"	(4) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
32'-0"		4,040 lbs.	≤ 12'-0"	10'-0"	W14x48	W14x43	9'-0"	3'-0"	29'-0"	1 1/2"	24"	24"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
		5,380 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	30'-0"	1 1/2"	24"	30"	3/8"	(4) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		6,720 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-6"	3'-0"	32'-0"	2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
32'-0"		8,070 lbs.	≤ 24'-0"	10'-0"	W18x97	W18x97	14'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		9,400 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	15'-0"	3'-0"	33'-0"	2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.105	3"	2"	30"
		3,080 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/2"	20"	20"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
32'-0"		4,610 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-6"	3'-0"	30'-0"	1 1/2"	24"	24"	3/8"	(6) 1 1/2" ∅	F1554-GR.55	2 1/4"	2"	30"
		6,150 lbs.	≤ 16'-0"	12'-0"	W16x87	W16x67	11'-0"	3'-0"	32'-0"	1 1/2"	24"	30"	3/8"	(4) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
		7,560 lbs.	≤ 20'-0"	12'-0"	W18x86	W18x76	13'-0"	3'-6"	32'-0"	1 1/2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	30"
32'-0"		9,220 lbs.	≤ 24'-0"	12'-0"	W18x119	W18x106	14'-0"	4'-0"	33'-0"	2"	24"	30"	CIP	(6) 1 1/2" ∅	F1554-GR.55	3"	2"	36"
		10,750 lbs.	≤ 28'-0"	12'-0"	W18x143	W18x130	15'-0"	4'-0"	33'-0"	2 1/4"	30"	36"	CIP	(6) - 2" ∅	F1554-GR.55	4"	2 1/2"	36"

SECTION A-A  
COLUMN ORIENTATION

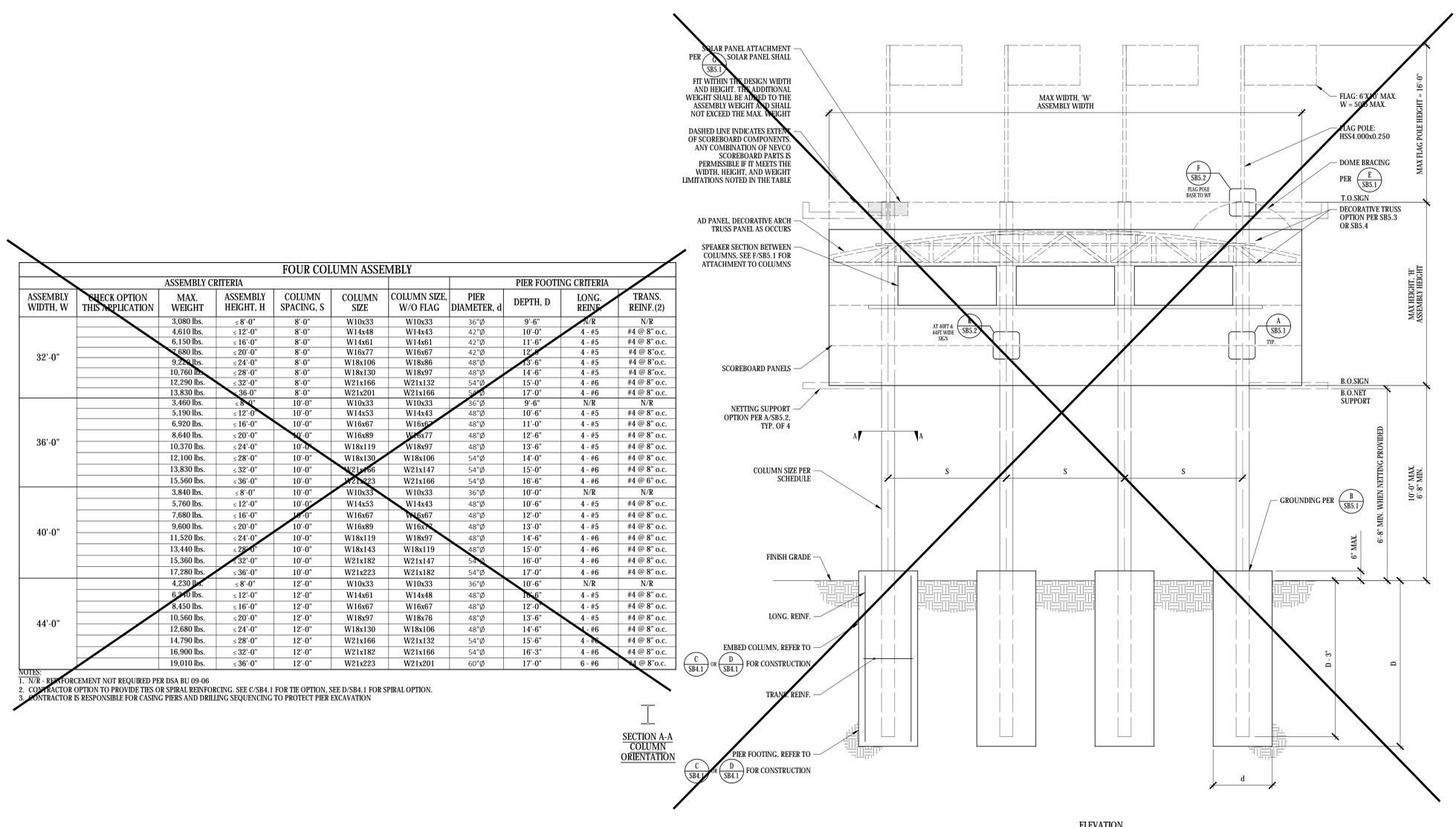


**THREE COLUMN SCOREBOARD INSTALLATION**

N.T.S.



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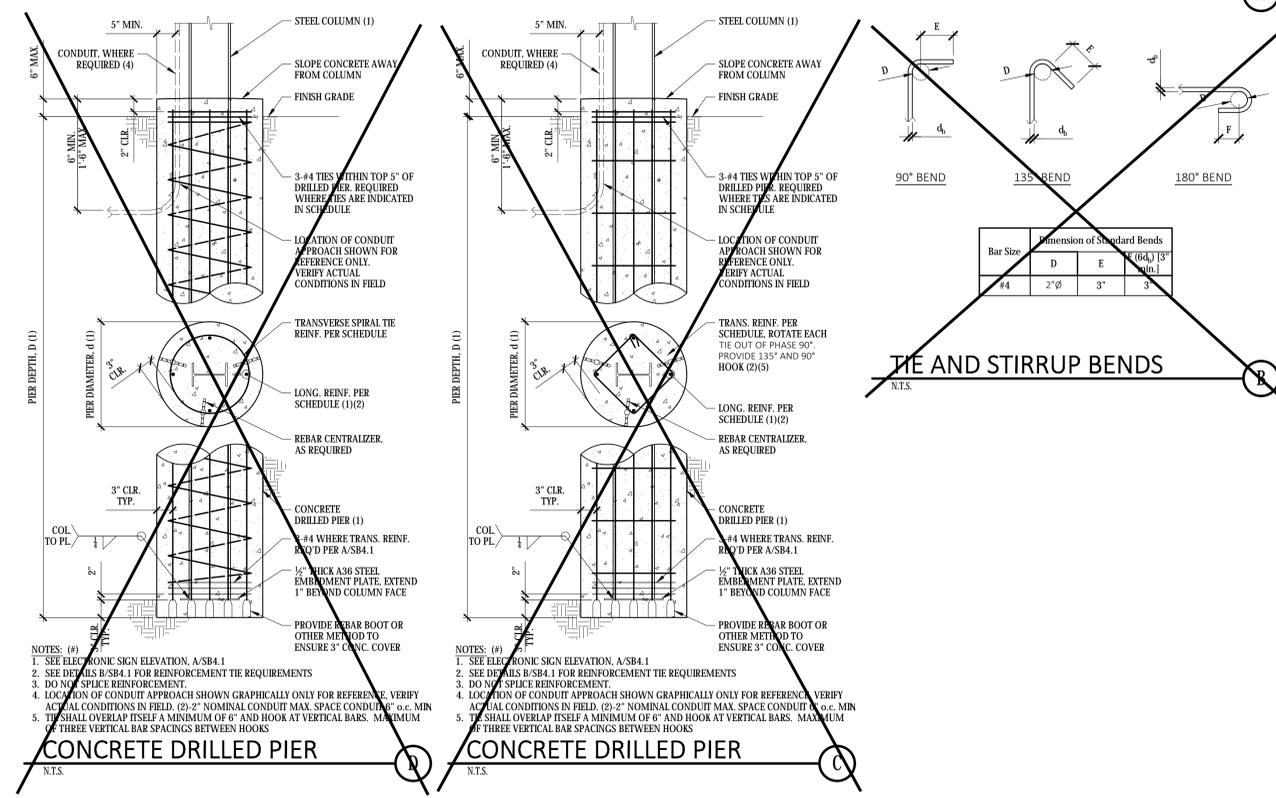


**FOUR COLUMN ASSEMBLY**

ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA			PIER FOOTING CRITERIA					
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (2)
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R	N/R
		4,610 lbs.	≤ 12'-0"	8'-0"	W14x48	W14x43	42"Ø	10'-0"	4-#5	#4 @ 8" o.c.
		6,150 lbs.	≤ 16'-0"	8'-0"	W14x61	W14x61	42"Ø	11'-6"	4-#5	#4 @ 8" o.c.
		7,680 lbs.	≤ 20'-0"	8'-0"	W16x77	W16x67	42"Ø	12'-6"	4-#5	#4 @ 8" o.c.
		9,220 lbs.	≤ 24'-0"	8'-0"	W18x106	W18x86	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.
36'-0"		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x97	48"Ø	14'-6"	4-#5	#4 @ 8" o.c.
		12,290 lbs.	≤ 32'-0"	8'-0"	W21x166	W21x132	54"Ø	15'-0"	4-#6	#4 @ 8" o.c.
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	54"Ø	17'-0"	4-#6	#4 @ 8" o.c.
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	9'-6"	N/R	N/R
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5	#4 @ 8" o.c.
40'-0"		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	11'-0"	4-#5	#4 @ 8" o.c.
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	12'-6"	4-#5	#4 @ 8" o.c.
		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	54"Ø	14'-0"	4-#6	#4 @ 8" o.c.
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	54"Ø	15'-0"	4-#6	#4 @ 8" o.c.
44'-0"		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	54"Ø	16'-6"	4-#6	#4 @ 6" o.c.
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	36"Ø	10'-0"	N/R	N/R
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	48"Ø	10'-6"	4-#5	#4 @ 8" o.c.
		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5	#4 @ 8" o.c.
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	48"Ø	13'-0"	4-#5	#4 @ 8" o.c.
	11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	48"Ø	14'-0"	4-#6	#4 @ 8" o.c.	
	13,440 lbs.	≤ 28'-0"	10'-0"	W18x143	W18x119	48"Ø	15'-0"	4-#6	#4 @ 8" o.c.	
	15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	54"Ø	16'-0"	4-#6	#4 @ 8" o.c.	
	17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	54"Ø	17'-0"	4-#6	#4 @ 8" o.c.	
	4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	36"Ø	10'-6"	N/R	N/R	
	6,240 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	48"Ø	11'-6"	4-#5	#4 @ 8" o.c.	
	8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	48"Ø	12'-0"	4-#5	#4 @ 8" o.c.	
	10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	48"Ø	13'-6"	4-#5	#4 @ 8" o.c.	
	12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	48"Ø	14'-6"	4-#6	#4 @ 8" o.c.	
	14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	54"Ø	15'-6"	4-#6	#4 @ 8" o.c.	
	16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	54"Ø	16'-3"	4-#6	#4 @ 8" o.c.	
	19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	60"Ø	17'-0"	6-#6	#4 @ 8" o.c.	

NOTES:  
 1. N/R - REINFORCEMENT NOT REQUIRED PER USA BU 09-06  
 2. CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.1 FOR THE OPTION. SEE D/SB4.1 FOR SPIRAL OPTION.  
 3. CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

**FOUR COLUMN SCOREBOARD INSTALLATION**



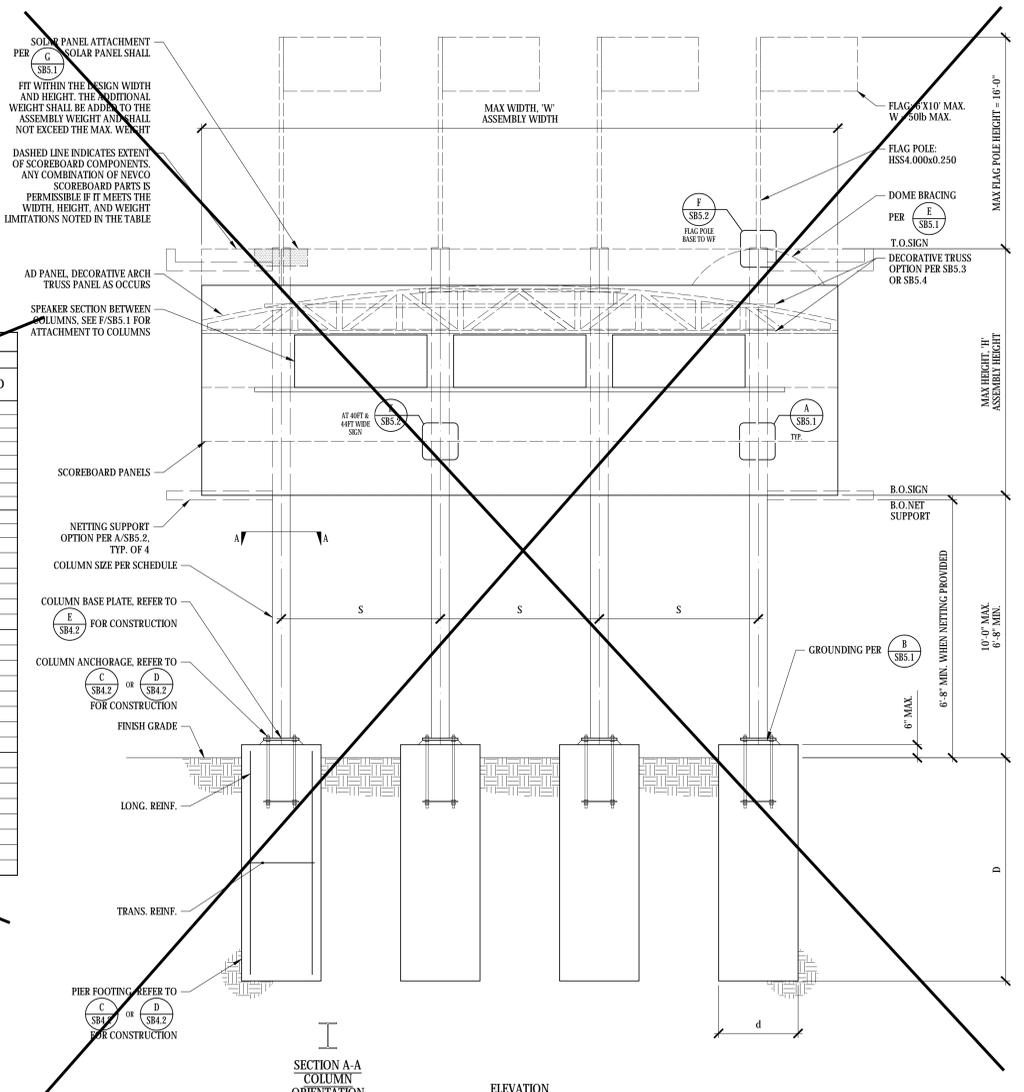
**CONCRETE DRILLED PIER**

**CONCRETE DRILLED PIER**

N.T.S.

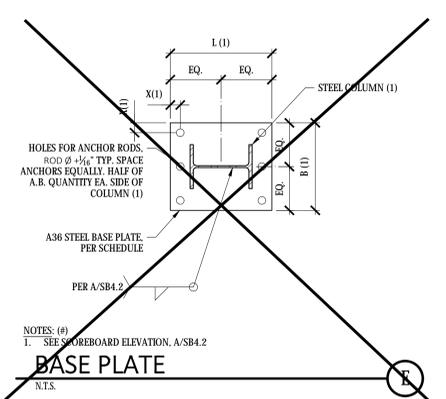
ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		PIER FOOTING CRITERIA				BASE PLATE				ANCHOR RODS						
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	PIER DIAMETER, d	DEPTH, D	LONG. REINF.	TRANS. REINF. (1)	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
32'-0"	3,080 lbs.	± 8'-0"	8'-0"	W10x33	W10x33	36"φ	9'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	20"	20"	3/16"	(4) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	4,610 lbs.	± 16'-0"	8'-0"	W14x48	W14x43	42"φ	10'-0"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/16"	(4) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	6,150 lbs.	± 24'-0"	8'-0"	W18x77	W18x77	48"φ	11'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/16"	(6) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	7,680 lbs.	± 24'-0"	8'-0"	W18x106	W18x86	48"φ	13'-6"	14-#9	#4 @ 6" o.c.	1 1/2"	24"	30"	3/16"	(6) 1 1/2"φ	F1554-GR.55	3"	2"	70"
	9,220 lbs.	± 24'-0"	8'-0"	W18x130	W18x97	48"φ	14'-6"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.105	3"	2"	70"
	10,760 lbs.	± 32'-0"	8'-0"	W21x147	W21x132	54"φ	15'-0"	18-#9	#4 @ 6" o.c.	2"	30"	36"	CIP	(6) 2"φ	F1554-GR.55	4"	2 1/2"	70"
36'-0"	13,830 lbs.	± 36'-0"	8'-0"	W21x201	W21x166	54"φ	17'-0"	22-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	70"
	3,460 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"φ	9'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	20"	20"	3/16"	(4) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	5,190 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"φ	10'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/16"	(4) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	6,920 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"φ	11'-0"	10-#8	#4 @ 6" o.c.	1 1/2"	24"	30"	3/16"	(4) 1 1/2"φ	F1554-GR.55	3"	2"	64"
	8,640 lbs.	± 20'-0"	10'-0"	W18x89	W18x77	48"φ	12'-6"	10-#8	#4 @ 6" o.c.	1 1/2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.55	3"	2"	64"
	10,370 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"φ	14'-6"	14-#9	#4 @ 6" o.c.	1 1/2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.55	3"	2"	64"
40'-0"	12,100 lbs.	± 28'-0"	10'-0"	W18x130	W18x106	54"φ	14'-0"	14-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	3/16"	(6) 2"φ	F1554-GR.55	4"	2 1/2"	70"
	13,830 lbs.	± 32'-0"	10'-0"	W21x166	W21x147	54"φ	15'-0"	18-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.55	4"	2 1/2"	70"
	15,560 lbs.	± 36'-0"	10'-0"	W21x223	W21x166	54"φ	16'-6"	22-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.55	4"	2 1/2"	70"
	3,840 lbs.	± 8'-0"	10'-0"	W10x33	W10x33	36"φ	10'-0"	8-#8	#4 @ 6" o.c.	1 1/2"	20"	20"	3/16"	(6) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	5,760 lbs.	± 12'-0"	10'-0"	W14x53	W14x43	48"φ	10'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/16"	(6) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
	7,680 lbs.	± 16'-0"	10'-0"	W16x67	W16x67	48"φ	12'-0"	10-#8	#4 @ 6" o.c.	1 1/2"	24"	30"	3/16"	(6) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
44'-0"	9,600 lbs.	± 20'-0"	10'-0"	W16x89	W16x77	48"φ	15'-0"	12-#8	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.55	3"	2"	64"
	11,520 lbs.	± 24'-0"	10'-0"	W18x119	W18x97	48"φ	14'-6"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.105	3"	2"	64"
	13,440 lbs.	± 28'-0"	10'-0"	W18x143	W18x119	48"φ	15'-0"	14-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	64"
	15,360 lbs.	± 32'-0"	10'-0"	W21x182	W21x147	54"φ	16'-0"	22-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	70"
	17,280 lbs.	± 36'-0"	10'-0"	W21x223	W21x182	54"φ	17'-0"	27-#9	#4 @ 6" o.c.	3"	30"	42"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	70"
	4,230 lbs.	± 8'-0"	12'-0"	W10x33	W10x33	36"φ	10'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	20"	20"	3/16"	(4) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"
6,340 lbs.	± 12'-0"	12'-0"	W14x61	W14x48	48"φ	10'-6"	8-#8	#4 @ 6" o.c.	1 1/2"	24"	24"	3/16"	(6) 1 1/2"φ	F1554-GR.55	2 1/2"	2"	64"	
8,450 lbs.	± 16'-0"	12'-0"	W16x67	W16x67	48"φ	12'-0"	10-#8	#4 @ 6" o.c.	1 1/2"	24"	30"	3/16"	(6) 1 1/2"φ	F1554-GR.55	3"	2"	64"	
10,560 lbs.	± 20'-0"	12'-0"	W18x77	W18x76	48"φ	13'-6"	12-#8	#4 @ 6" o.c.	1 1/2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.55	3"	2"	64"	
12,680 lbs.	± 24'-0"	12'-0"	W18x130	W18x106	48"φ	14'-6"	14-#9	#4 @ 6" o.c.	2"	24"	30"	CIP	(6) 1 1/2"φ	F1554-GR.105	3"	2"	64"	
14,790 lbs.	± 28'-0"	12'-0"	W21x166	W21x132	54"φ	15'-6"	18-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	70"	
16,900 lbs.	± 32'-0"	12'-0"	W21x182	W21x166	54"φ	16'-3"	22-#9	#4 @ 6" o.c.	2 1/2"	30"	36"	CIP	(6) 2"φ	F1554-GR.105	4"	2 1/2"	70"	
19,010 lbs.	± 36'-0"	12'-0"	W21x223	W21x201	60"φ	17'-0"	27-#9	#4 @ 6" o.c.	3"	30"	42"	CIP	(10) 2"φ	F1554-GR.105	4"	2 1/2"	70"	

NOTES: (1) CONTRACTOR OPTION TO PROVIDE TIES OR SPIRAL REINFORCING. SEE C/SB4.2 FOR THE OPTION. SEE D/SB4.2 FOR SPIRAL OPTION.  
 (2) CONTRACTOR IS RESPONSIBLE FOR CASING PIERS AND DRILLING SEQUENCING TO PROTECT PIER EXCAVATION

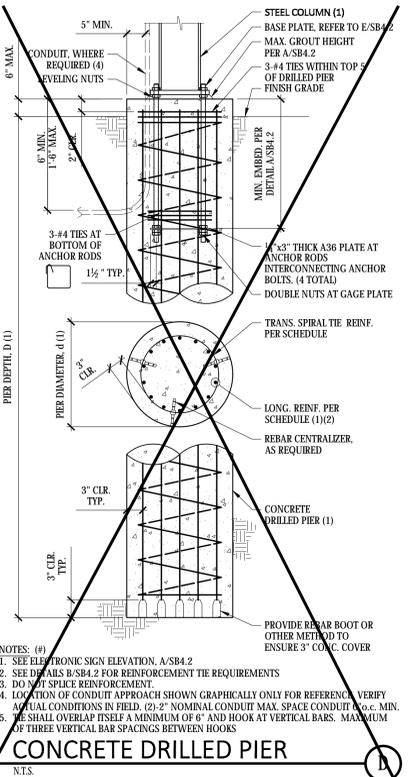


SECTION A-A COLUMN ORIENTATION ELEVATION

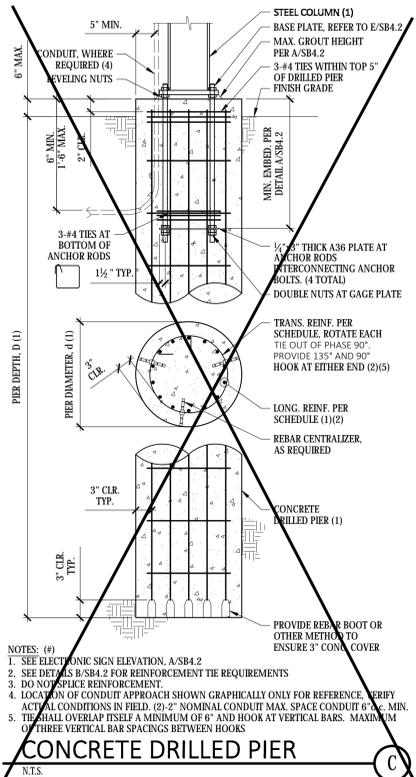
FOUR COLUMN SCOREBOARD INSTALLATION



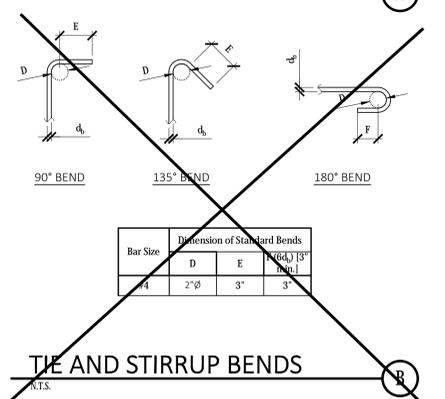
BASE PLATE



CONCRETE DRILLED PIER



CONCRETE DRILLED PIER



TIE AND STIRRUP BENDS

APPLICATION # 02-122088

IDENTIFICATION STAMP  
 DIV. OF THE STATE ARCHITECT  
 APP: 02-122088 INC.  
 REVIEWED FOR  
 SS  FLS  ACS   
 DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL E. RANOLD  
 No. 5485  
 STRUCTURAL  
 STATE OF CALIFORNIA  
 EXPIRES 08.09.2023

PC SEOR REAL

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THANK YOU FOR YOUR INTEREST IN NEVCO SCOREBOARD PRODUCTS

NEVCO  
 301 East Harris Avenue, Greenville, Illinois 62246  
 Phone: 619.664.0960  
 www.nevco.com

APPROVED  
 DIV. OF THE STATE ARCHITECT  
 APP: 04-122317 PC  
 REVIEWED FOR  
 SS  FLS  ACS  CG   
 DATE: 09/20/2022

PRE-CHECK (PC) DOCUMENT  
 CODE: 2022

A separate project application for construction is required.

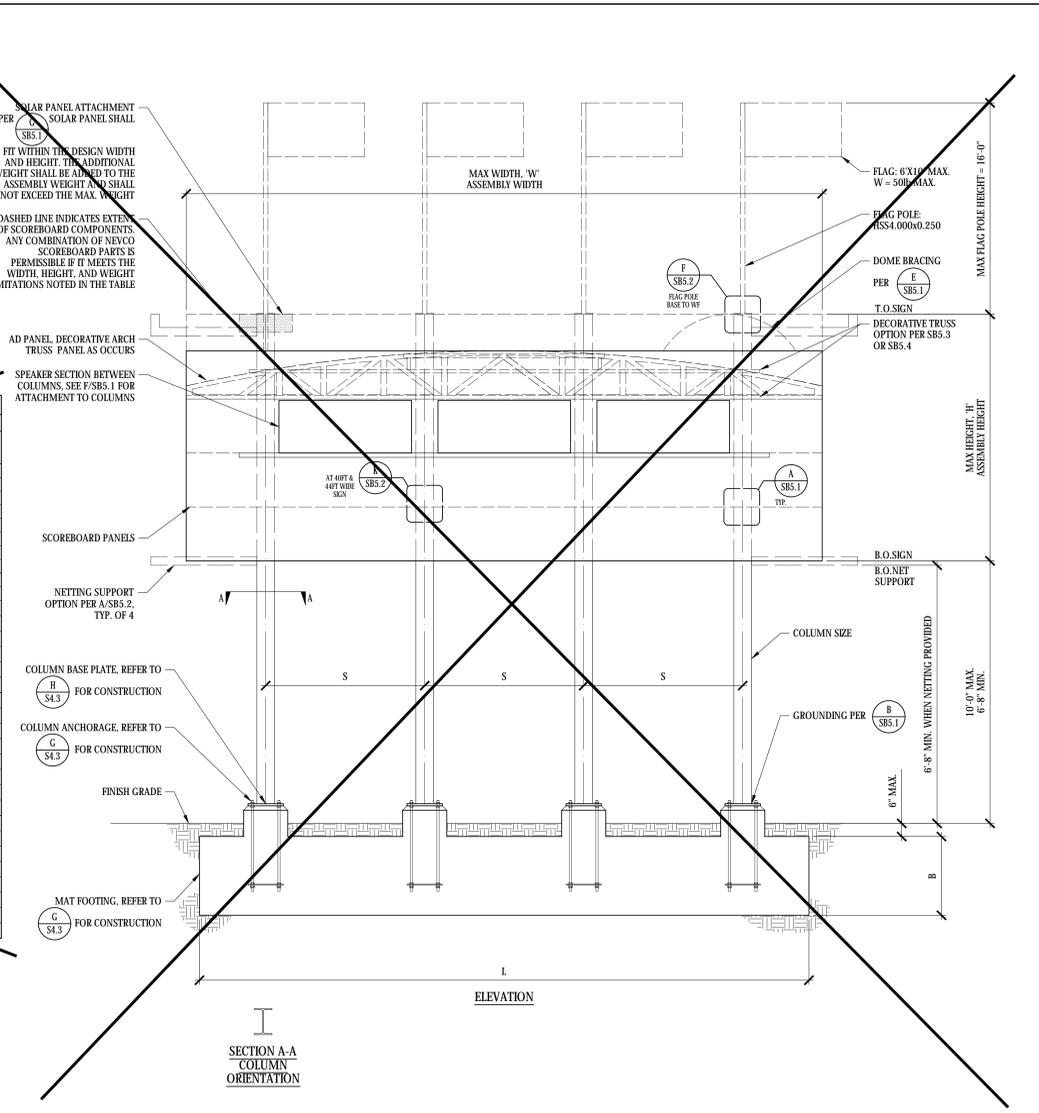
REGISTERED PROFESSIONAL ENGINEER  
 MICHAEL E. RANOLD  
 No. 52386  
 STRUCTURAL  
 STATE OF CALIFORNIA  
 EXPIRES 3-31-24

FOUR COLUMN CAISSON - BOLTED

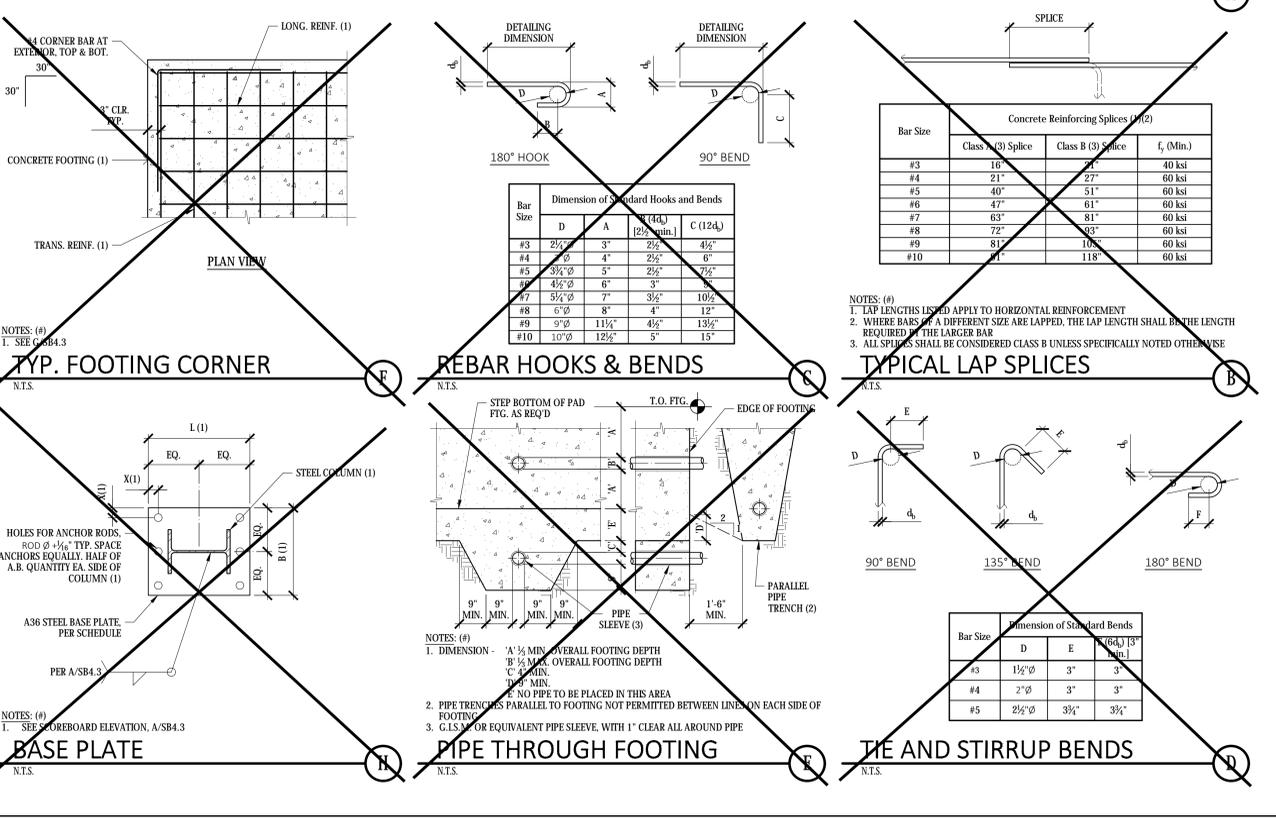
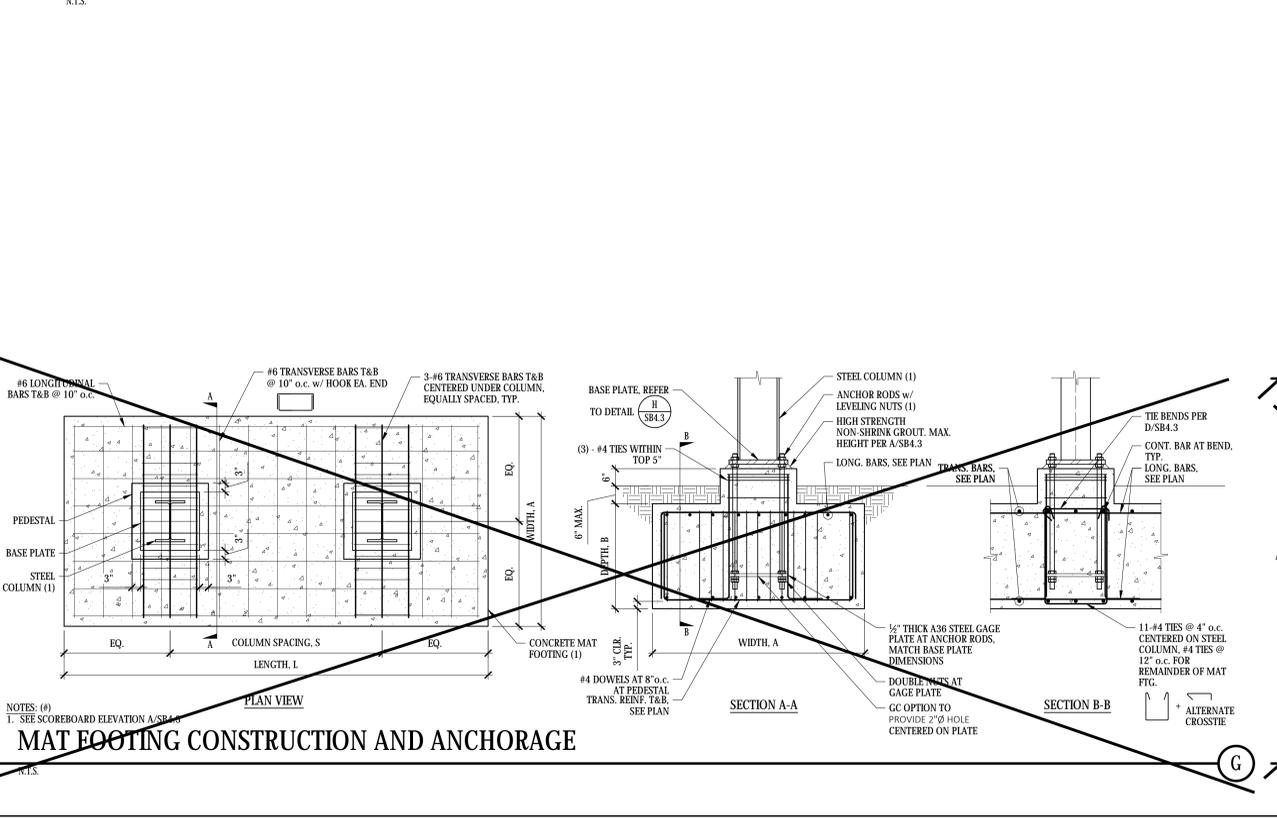
SHEET INFORMATION  
 DATE: 08.09.2023  
 DRAWN: JMK  
 CHECKED: MEP  
 SSG JOB #: S23109  
 SHEET: SB4.2

**FOUR COLUMN ASSEMBLY**

ASSEMBLY WIDTH, W	CHECK OPTION THIS APPLICATION	ASSEMBLY CRITERIA		MAT FOOTING CRITERIA				BASE PLATE			ANCHOR RODS							
		MAX. WEIGHT	ASSEMBLY HEIGHT, H	COLUMN SPACING, S	COLUMN SIZE	COLUMN SIZE, W/O FLAG	WIDTH, A	DEPTH, B	LENGTH, L	THICKNESS, t	WIDTH, B	LENGTH, L	WELD	QUANTITY & DIAMETER	GRADE	EDGE DISTANCE, X	GROUT HEIGHT	EMBED
32'-0"		3,080 lbs.	≤ 8'-0"	8'-0"	W10x33	W10x33	8'-0"	3'-0"	28'-0"	1 1/4"	20"	20"	3/8"	(4) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		4,610 lbs.	≤ 16'-0"	8'-0"	W14x48	W14x48	9'-0"	3'-0"	30'-0"	1 1/4"	24"	24"	3/8"	(4) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		6,150 lbs.	≤ 24'-0"	8'-0"	W14x61	W14x61	10'-6"	3'-0"	35'-0"	1 1/2"	24"	24"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		7,690 lbs.	≤ 32'-0"	8'-0"	W16x77	W16x77	12'-6"	3'-0"	36'-0"	1 3/4"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
		9,230 lbs.	≤ 40'-0"	8'-0"	W18x106	W18x106	14'-0"	3'-0"	39'-0"	1 3/4"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
36'-0"		10,760 lbs.	≤ 28'-0"	8'-0"	W18x130	W18x130	15'-0"	3'-6"	39'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105	3"	2"	30"
		12,290 lbs.	≤ 32'-0"	8'-0"	W21x176	W21x132	17'-0"	4'-0"	42'-0"	2"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 36'-0"	8'-0"	W21x201	W21x166	18'-0"	4'-0"	46'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"
		3,460 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	7'-6"	3'-0"	36'-0"	1 1/4"	20"	20"	3/8"	(4) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		5,190 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	36'-0"	1 1/4"	24"	24"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
40'-0"		6,920 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	36'-0"	1 1/2"	24"	30"	3/8"	(4) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
		8,640 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	12'-0"	3'-0"	40'-0"	1 1/2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
		10,370 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	1 3/4"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
		12,100 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x106	14'-0"	4'-0"	40'-0"	1 3/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55	4"	2 1/2"	36"
		13,830 lbs.	≤ 32'-0"	10'-0"	W21x166	W21x147	16'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.55	4"	2 1/2"	36"
44'-0"		15,560 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x166	18'-0"	4'-0"	40'-0"	2 1/4"	30"	36"	CIP	(8) - 2" Ø	F1554 - GR.55	4"	2 1/2"	36"
		3,840 lbs.	≤ 8'-0"	10'-0"	W10x33	W10x33	8'-0"	3'-0"	38'-0"	1 1/2"	20"	20"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		5,760 lbs.	≤ 12'-0"	10'-0"	W14x53	W14x43	9'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		7,680 lbs.	≤ 16'-0"	10'-0"	W16x67	W16x67	11'-0"	3'-0"	38'-0"	1 1/2"	24"	24"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
		9,600 lbs.	≤ 20'-0"	10'-0"	W16x89	W16x77	13'-0"	3'-0"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"
44'-0"		11,520 lbs.	≤ 24'-0"	10'-0"	W18x119	W18x97	14'-0"	3'-6"	40'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105	3"	2"	30"
		13,440 lbs.	≤ 28'-0"	10'-0"	W18x130	W18x119	14'-0"	4'-0"	42'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"
		15,360 lbs.	≤ 32'-0"	10'-0"	W21x182	W21x147	16'-0"	4'-0"	44'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"
		17,280 lbs.	≤ 36'-0"	10'-0"	W21x223	W21x182	18'-0"	4'-0"	46'-0"	3"	30"	42"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"
		4,230 lbs.	≤ 8'-0"	12'-0"	W10x33	W10x33	7'-0"	3'-0"	42'-0"	1 1/2"	20"	20"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"
	6,340 lbs.	≤ 12'-0"	12'-0"	W14x61	W14x48	9'-0"	3'-0"	43'-0"	1 1/2"	24"	24"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"	
	8,450 lbs.	≤ 16'-0"	12'-0"	W16x67	W16x67	10'-0"	3'-0"	43'-0"	1 1/2"	24"	30"	3/8"	(6) - 1 1/2" Ø	F1554 - GR.55	2 1/2"	2"	30"	
	10,560 lbs.	≤ 20'-0"	12'-0"	W18x97	W18x76	12'-0"	3'-6"	46'-0"	1 3/4"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.55	3"	2"	30"	
	12,680 lbs.	≤ 24'-0"	12'-0"	W18x130	W18x106	14'-0"	3'-6"	47'-0"	2"	24"	30"	CIP	(6) - 1 1/2" Ø	F1554 - GR.105	3"	2"	30"	
	14,790 lbs.	≤ 28'-0"	12'-0"	W21x166	W21x132	15'-0"	4'-0"	48'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	30"	
	16,900 lbs.	≤ 32'-0"	12'-0"	W21x182	W21x166	16'-0"	4'-0"	49'-0"	2 1/4"	30"	36"	CIP	(6) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"	
	19,010 lbs.	≤ 36'-0"	12'-0"	W21x223	W21x201	18'-0"	4'-0"	52'-0"	3"	30"	42"	CIP	(10) - 2" Ø	F1554 - GR.105	4"	2 1/2"	36"	



**FOUR COLUMN SCOREBOARD INSTALLATION**



APPLICATION # 02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC.  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

**SSG**  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. RANOLD  
No. 5485  
STRUCTURAL  
STATE OF CALIFORNIA  
08.09.2023

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**NEVCO**  
301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0560  
www.nevco.com

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DIV. OF THE STATE ARCHITECT  
APP: 04-122377 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2024

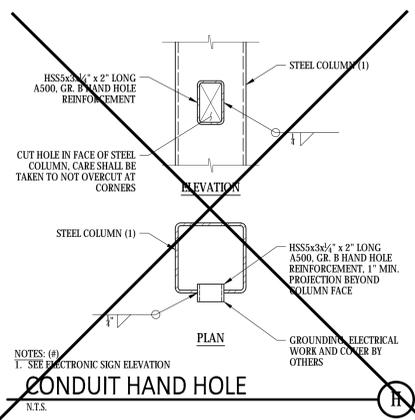
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CODE: 2022

A separate project application for construction is required.

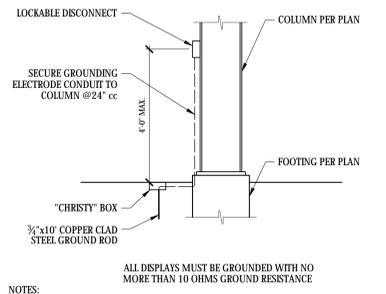
REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. RANOLD  
No. 52366  
STRUCTURAL  
STATE OF CALIFORNIA  
3-31-24

**FOUR COLUMN MAT FOOTING**

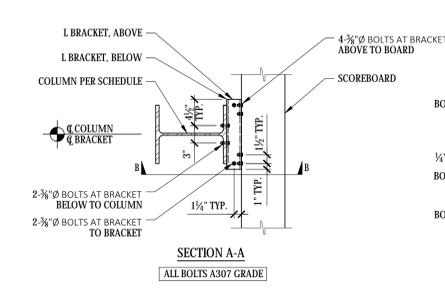
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DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB4.3



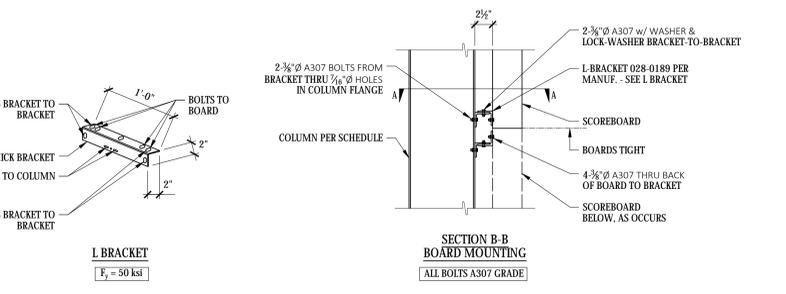
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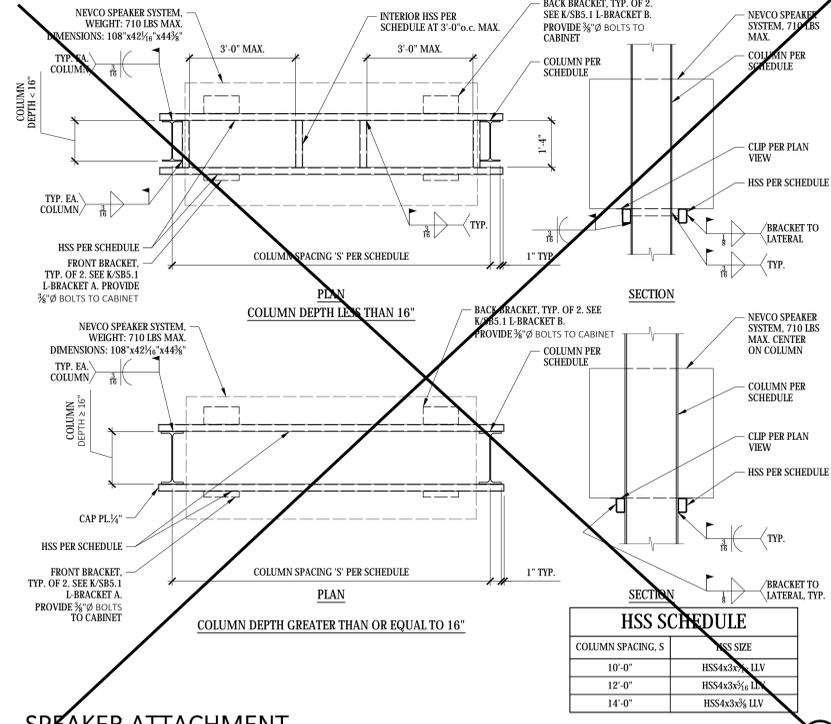
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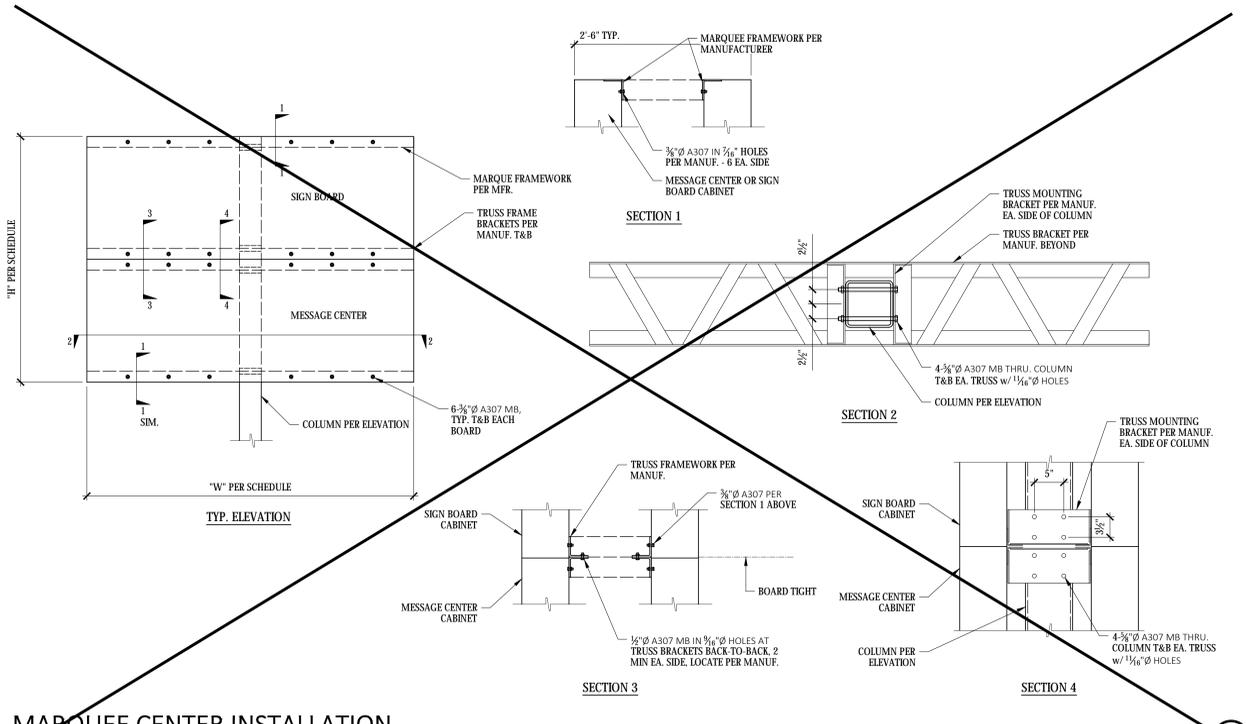
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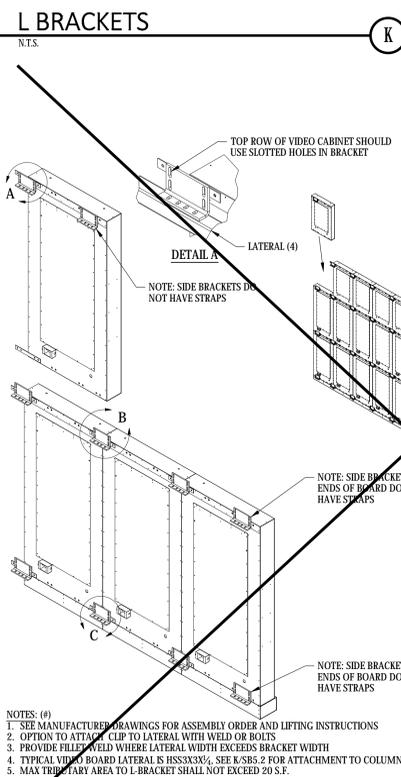
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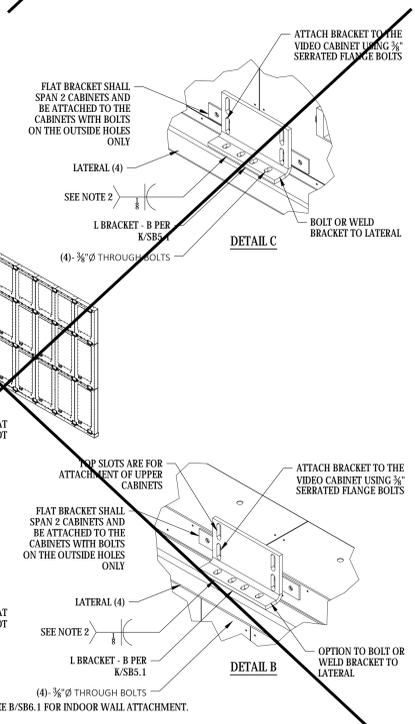
**SPEAKER ATTACHMENT**  
N.T.S.



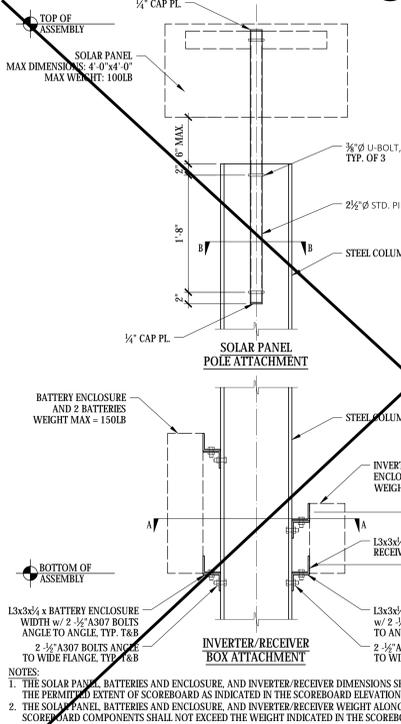
**MARQUEE CENTER INSTALLATION**  
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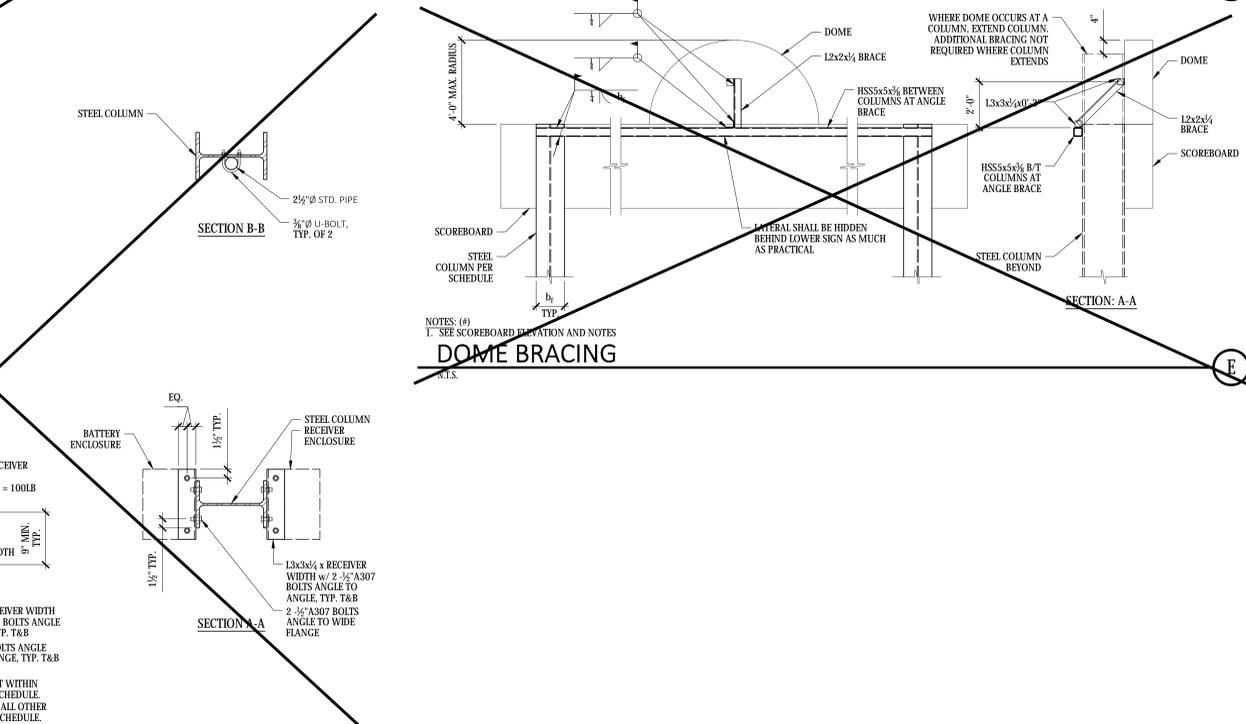
**L BRACKETS**  
N.T.S.



**VIDEO BOARD BRACKET ATTACHMENT**  
N.T.S.



**SOLAR PANEL/BATTERY/INVERTER/RECEIVER ATTACHMENT**  
N.T.S.



**DOME BRACING**  
N.T.S.

APPLICATION # 02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC.  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

**SSG**  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
STATE OF CALIFORNIA  
No. 5485  
DATE REISSUED: 08.09.2023

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**NEVCO**  
www.nevco.com  
301 East Harris Avenue, Greenville, Illinois 62246  
Phone: 619.664.0980

APPROVED  
DIV. OF THE STATE ARCHITECT  
APP: 04-122088 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2023

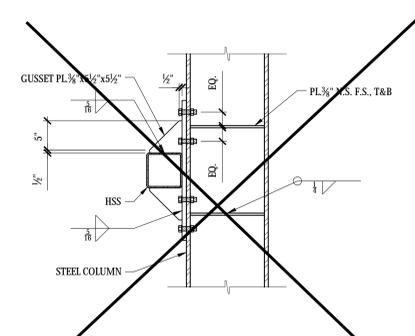
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CODE: 2022

A separate project application for construction is required.

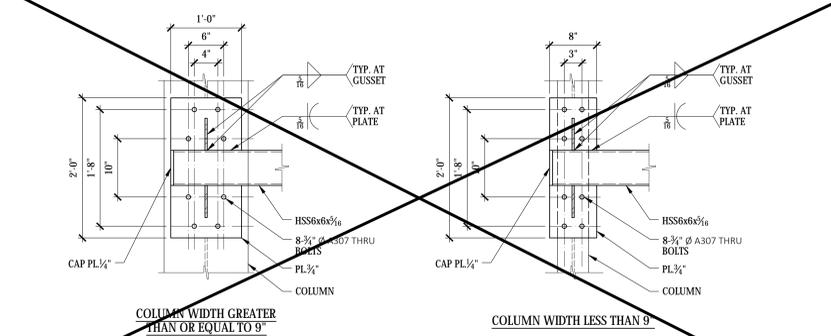
REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA REGISTERED PROFESSIONAL ENGINEER  
No. 52386  
EXPIRES 3-31-24  
DATE REISSUED: 08.09.2023

**ATTACHMENT DETAILS**

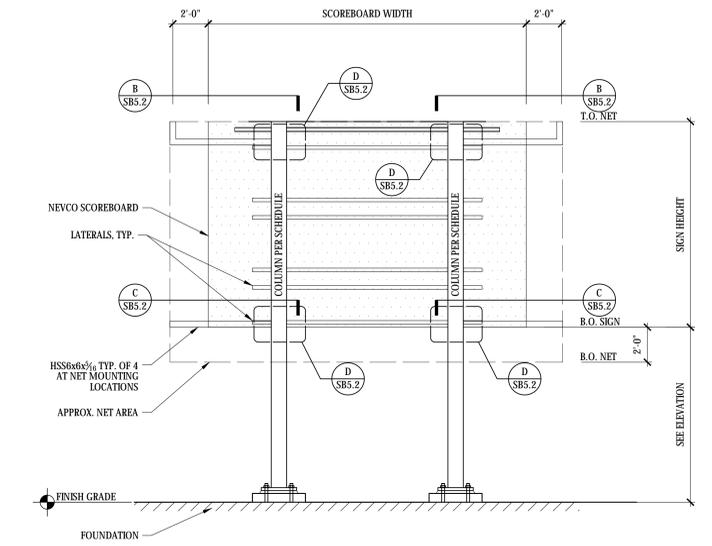
SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: **SB5.1**



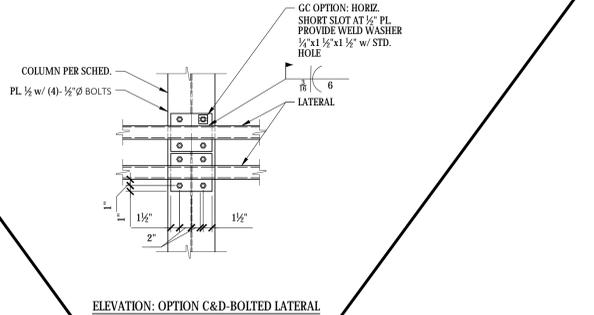
NET MOUNT PLATE  
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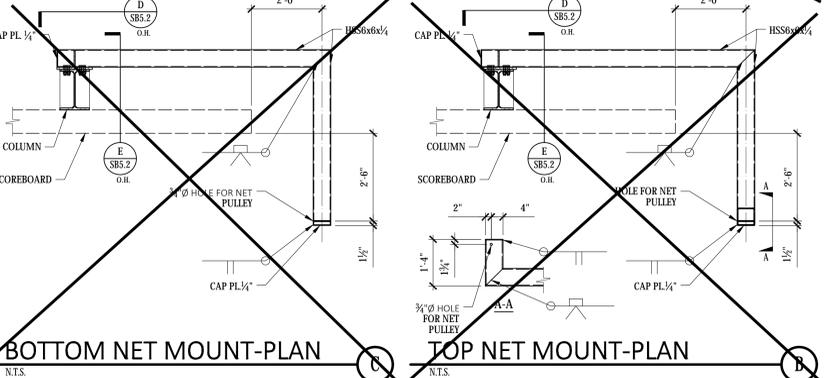
NET MOUNT PLATE  
COLUMN WIDTH GREATER THAN OR EQUAL TO 9"  
N.T.S.



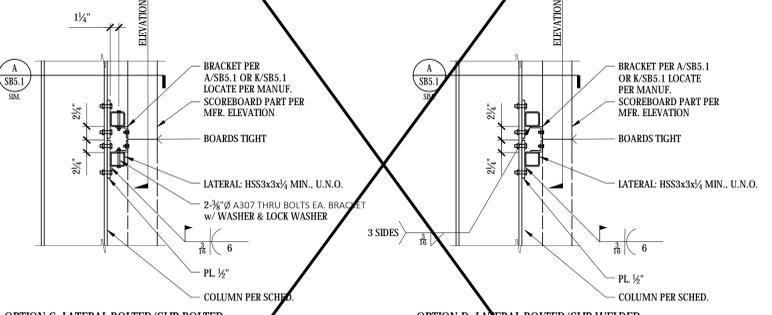
SCOREBOARD NETTING SUPPORTS  
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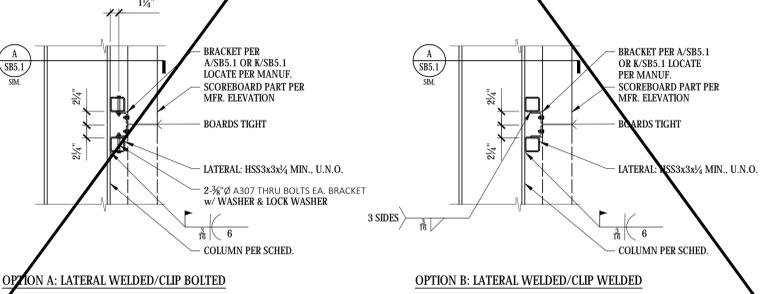
ELEVATION: OPTION C&D-BOLTED LATERAL  
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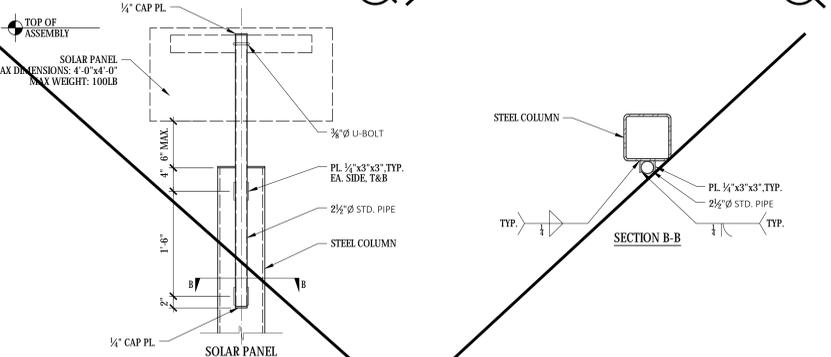
BOTTOM NET MOUNT-PLAN  
TOP NET MOUNT-PLAN  
N.T.S.



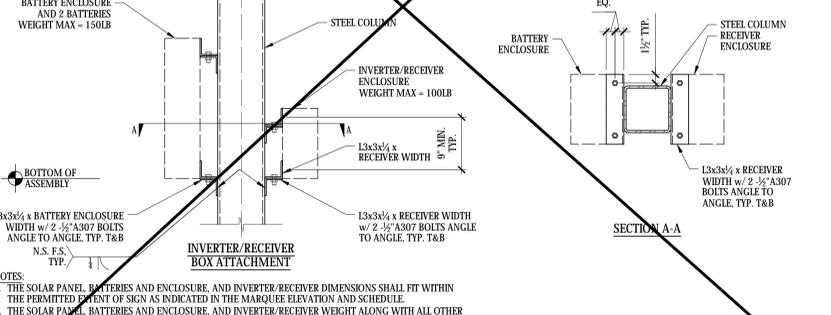
OPTION C: LATERAL BOLTED/CLIP BOLTED  
OPTION D: LATERAL BOLTED/CLIP WELDED  
N.T.S.



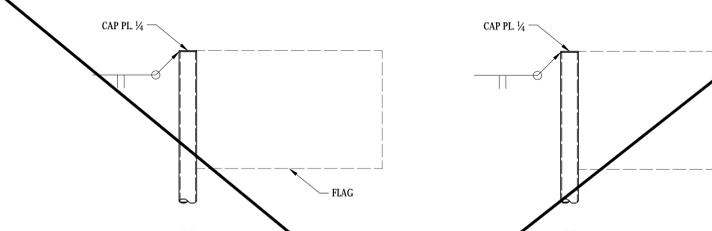
OPTION A: LATERAL WELDED/CLIP BOLTED  
OPTION B: LATERAL WELDED/CLIP WELDED  
N.T.S.



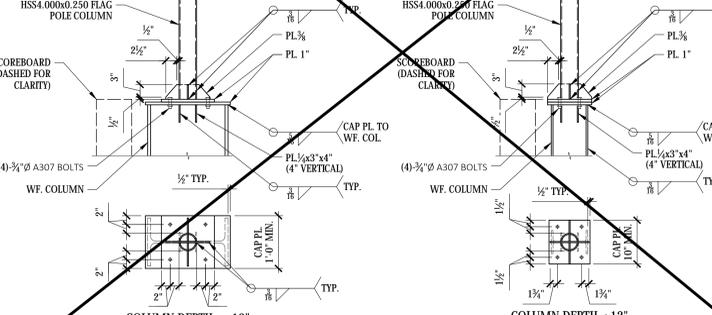
SOLAR PANEL POLE ATTACHMENT  
N.T.S.



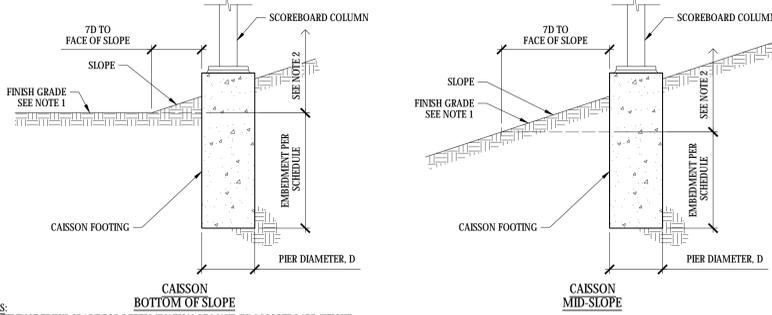
INVERTER/RECEIVER BOX ATTACHMENT  
N.T.S.



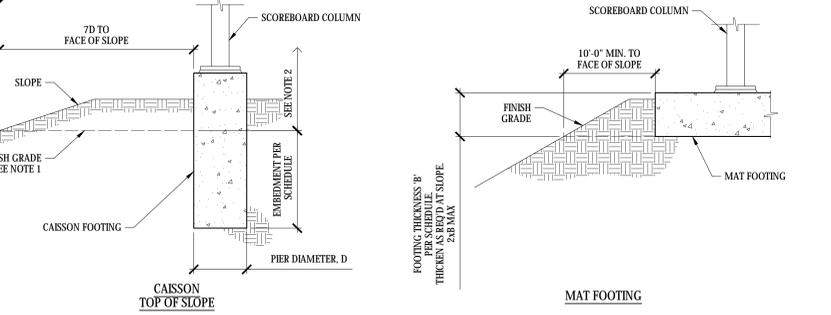
SECTION B-B  
N.T.S.



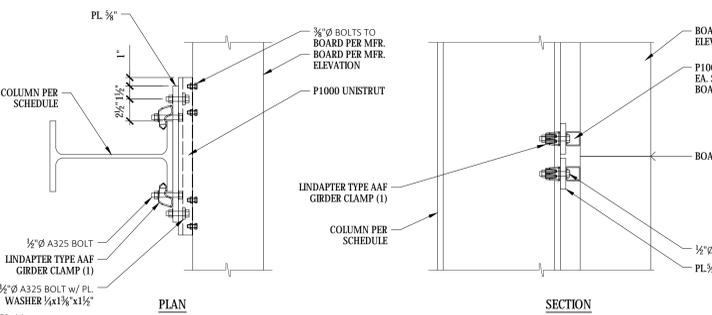
FLAG POLE COLUMN TO WF BEAM  
N.T.S.



DISTANCE TO SLOPE  
N.T.S.



SOLAR PANEL/BATTERY/INVERTER/RECEIVER ATTACHMENT  
N.T.S.



ALTERNATE BOARD ATTACHMENT  
N.T.S.

APPLICATION # 02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC.  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

SSG structural engineers

REGISTERED PROFESSIONAL ENGINEER  
MICHAEL E. RAYMOND  
No. 5485  
STATE OF CALIFORNIA  
EXPIRES 08.09.2023

PC SEOR REAL

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Phone: (618) 664-0980  
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APP: 04-122377 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2022

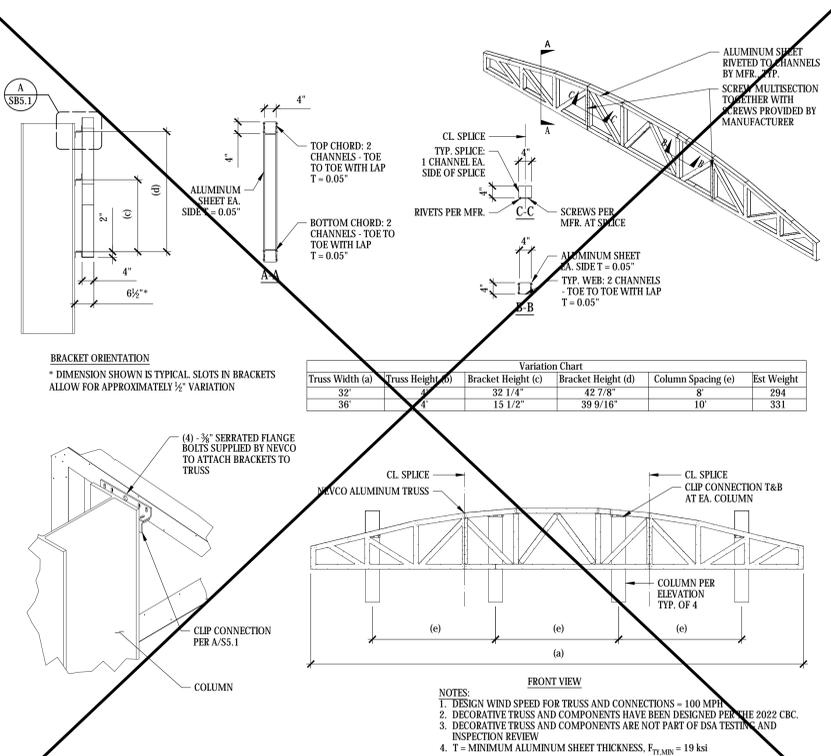
PRE-CHECK (PC) DOCUMENT CODE: 2022

A separate project application for construction is required.

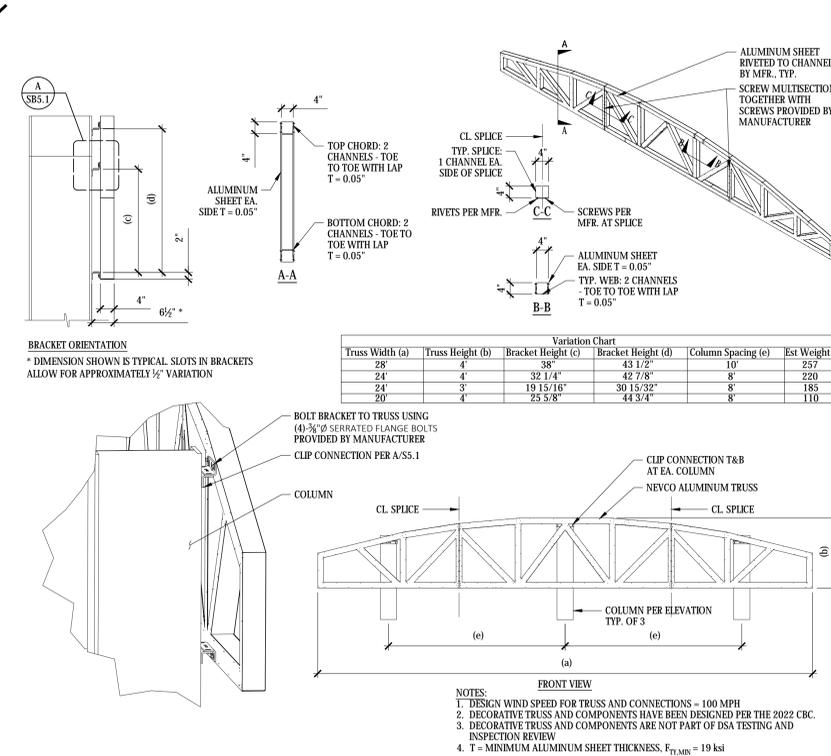
REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA REGISTERED PROFESSIONAL ENGINEER  
NO. 52386  
EXPIRES 03-31-24

OPTIONAL SCOREBOARD FEATURE ATTACHMENT DETAILS

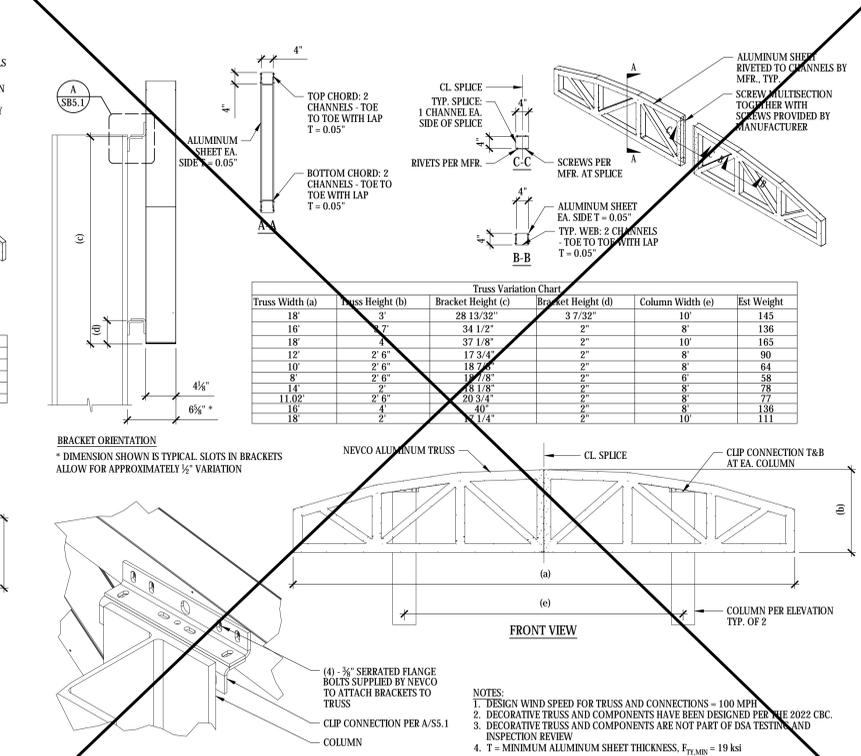
SHEET INFORMATION	
DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	SB5.2



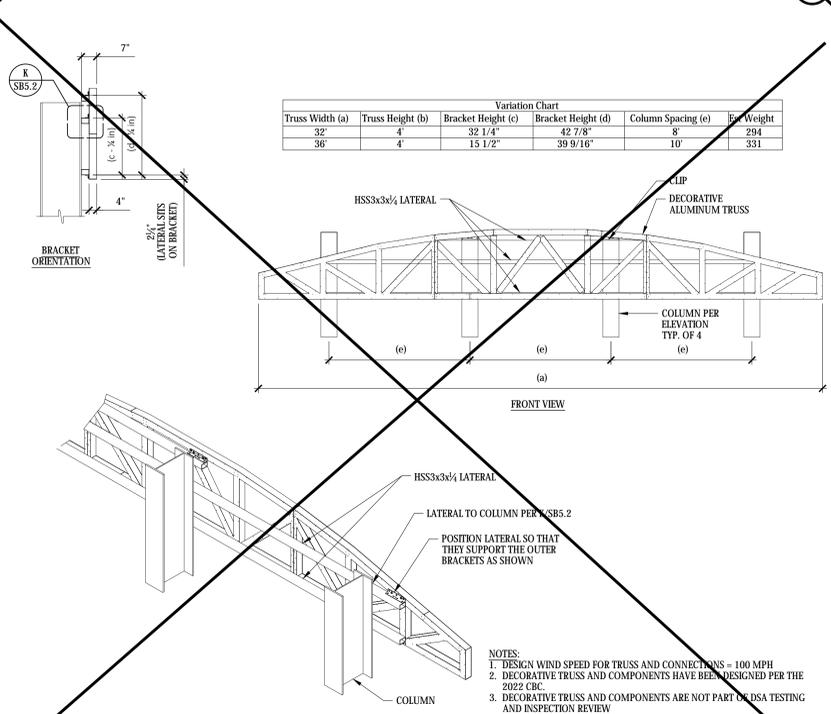
**INSTALLATION OF ALUMINUM TRUSS ON FOUR COLUMNS**



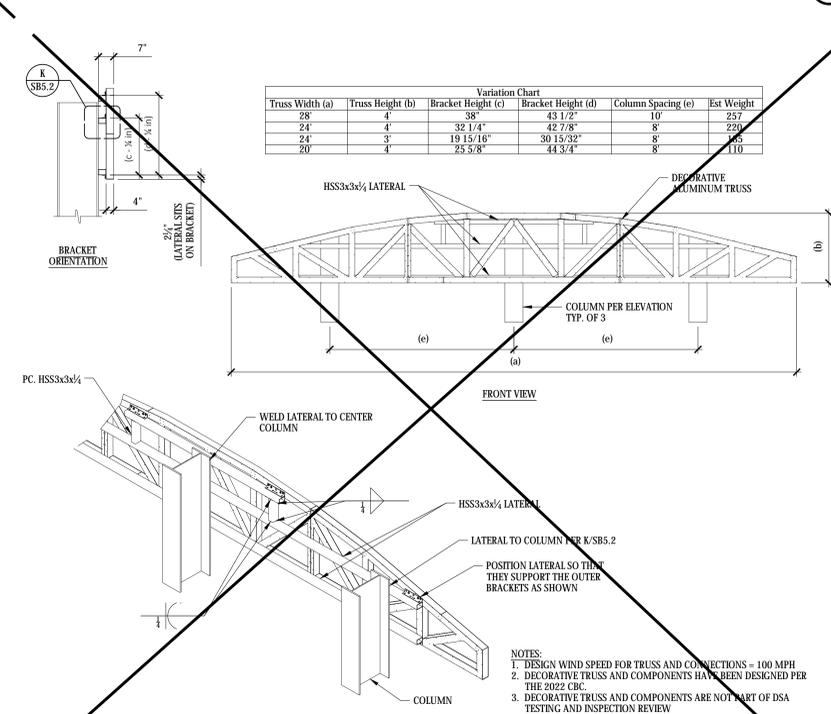
**INSTALLATION OF ALUMINUM TRUSS ON THREE COLUMNS**



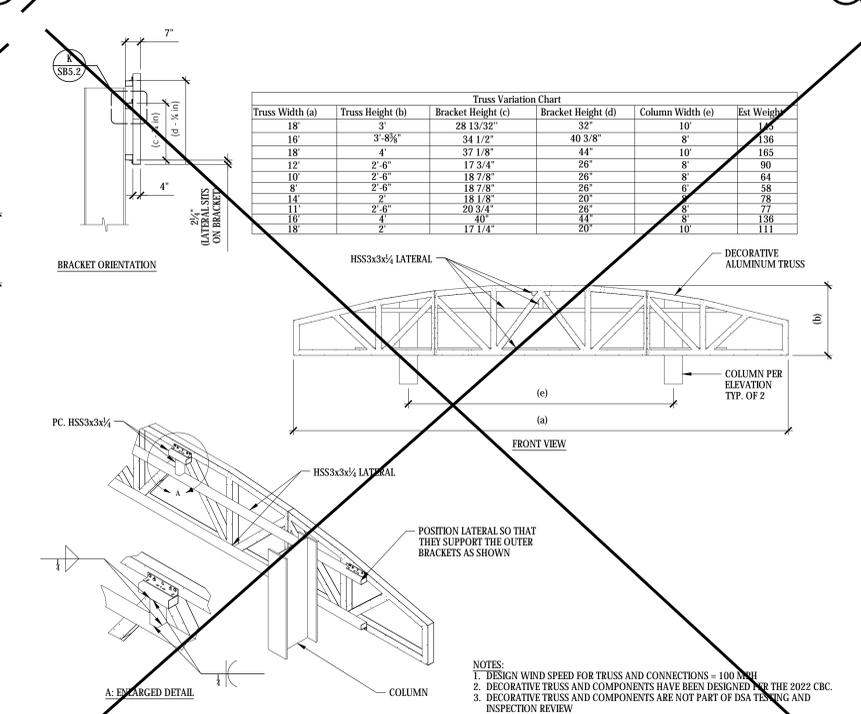
**INSTALLATION OF ALUMINUM TRUSS ON TWO COLUMNS**



**INSTALLATION OF ALUMINUM TRUSS ON FOUR COL. w/ LATERAL**



**INSTALLATION OF ALUMINUM TRUSS ON THREE COL. w/ LATERAL**



**INSTALLATION OF ALUMINUM TRUSS ON TWO COL. w/ LATERAL**

APPLICATION # 02-122088

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC.  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024

**SSG**  
structural engineers

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA No. 5485  
STRUCTURAL  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023

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THANK YOU FOR YOUR INTEREST IN NEVCO'S INNOVATIVE PRODUCTS

**NEVCO**

301 East Harris Avenue, Greenville, Illinois 62246  
Phone: (618) 664-0960  
www.nevco.com

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DIV. OF THE STATE ARCHITECT  
APP: 04-122377 PC  
REVIEWED FOR  
SS  FLS  ACS  CG   
DATE: 09/20/2023

PRE-CHECK (PC) DOCUMENT  
CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA No. 52366  
STRUCTURAL  
STATE OF CALIFORNIA  
DATE SIGNED: 08.09.2023

**DECORATIVE ALUMINUM TRUSS ATTACHMENT DETAILS**

SHEET INFORMATION  
DATE: 08.09.2023  
DRAWN: JMK  
CHECKED: MEP  
SSG JOB #: S23109  
SHEET: SB5.3

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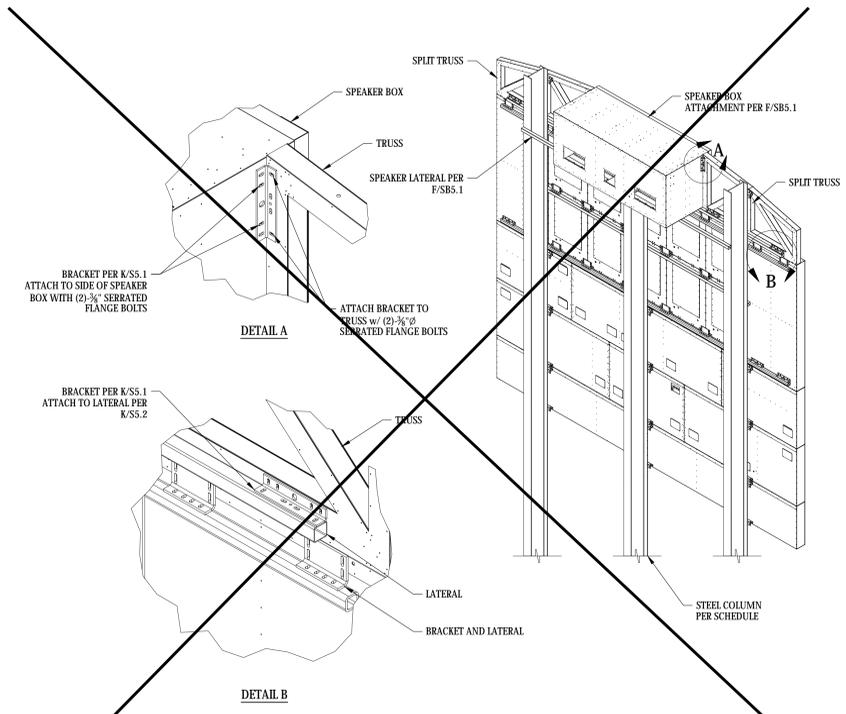
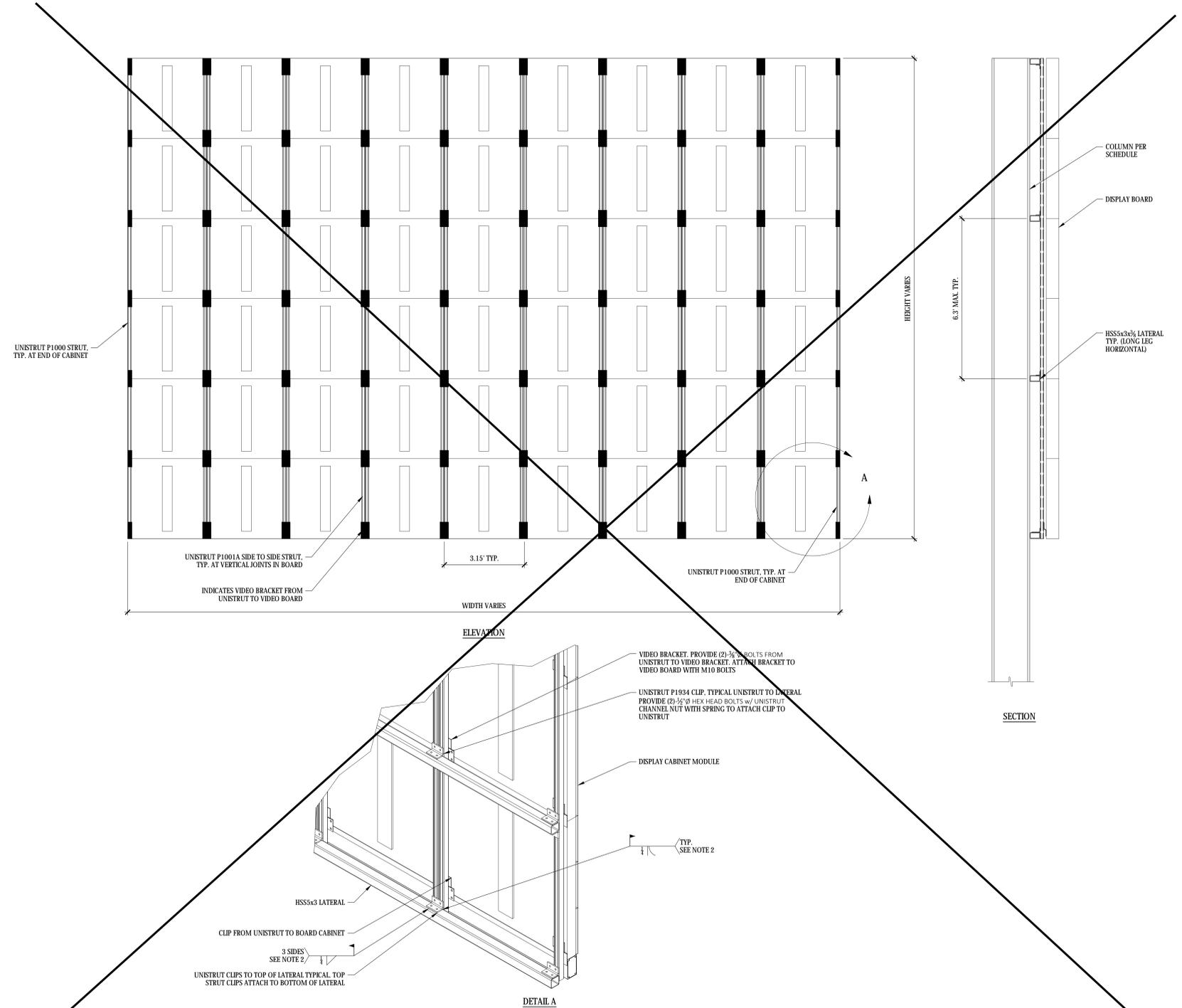
PRE-CHECK (PC) DOCUMENT  
 CODE: 2022

A separate project application for construction is required.

REGISTERED PROFESSIONAL ENGINEER  
 CALIFORNIA R. RANOLD  
 No. 52386  
 EXPIRES 3-31-24  
 STRUCTURAL

**ALUMINUM TRUSS ATTACHMENT DETAILS & 10mm VIDEO DISPLAY SUPPORT**

DATE	08.09.2023
DRAWN	JMK
CHECKED	MEP
SSG JOB #	S23109
SHEET	<b>SB5.4</b>



NOTES: (0)  
 1. DESIGN OF SUPPORTS AND CONNECTIONS IS BASED ON A VIDEO DISPLAY WITH A WEIGHT OF 8 LB/SQ. FT. TO DETERMINE TOTAL VIDEO DISPLAY WEIGHT CONTACT NEVCO SCOREBOARDS  
 2. ALTERNATE OPTION TO BOLT CLIP TO LATERAL WITH 1/2\"/>

NOTES:  
 1. DETAIL B IS APPLICABLE TO 3 COLUMN AND 4 COLUMN ASSEMBLIES  
**SPLIT DECORATIVE TRUSS FLANKING SPEAKER BOX**



### ELECTRICAL EQUIPMENT ANCHORAGE NOTES

ALL ELECTRICAL COMPONENTS SHALL BE ANCHORED AND INSTALLED PER THE DETAILS ON THE DSA APPROVED CONSTRUCTION DOCUMENTS. WHERE NO DETAIL IS INDICATED, THE FOLLOWING COMPONENTS SHALL BE ANCHORED OR BRACED TO MEET THE FORCE AND DISPLACEMENT REQUIREMENTS PRESCRIBED IN THE 2022 CBC, SECTIONS 1617A.1.18 THROUGH 1617A.1.26 AND ASCE 7-16, CHAPTERS 13, 26 AND 30:

- ALL PERMANENT EQUIPMENT AND COMPONENTS.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT THAT IS PERMANENTLY ATTACHED (E.G. HARD WIRED) TO THE BUILDING ELECTRICAL UTILITY SERVICE. "PERMANENTLY ATTACHED" SHALL INCLUDE ALL ELECTRICAL CONNECTIONS EXCEPT PLUGS FOR 120/220 VOLT RECEPTACLES HAVING A FLEXIBLE CABLE.
- TEMPORARY, MOVABLE OR MOBILE EQUIPMENT WHICH IS HEAVIER THAN 400 POUNDS OR HAS A CENTER OF MASS LOCATED 4 FEET OR MORE ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT IS REQUIRED TO BE RESTRAINED IN A MANNER APPROVED BY DSA.

THE FOLLOWING ELECTRICAL COMPONENTS SHALL BE POSITIVELY ATTACHED TO THE STRUCTURE BUT NEED NOT DEMONSTRATE DESIGN COMPLIANCE WITH THE REFERENCES NOTED ABOVE. THESE COMPONENTS SHALL HAVE FLEXIBLE CONNECTIONS PROVIDED BETWEEN THE COMPONENT AND ASSOCIATED CONDUIT. FLEXIBLE CONNECTIONS MUST ALLOW MOVEMENT IN BOTH TRANSVERSE AND LONGITUDINAL DIRECTIONS:

- COMPONENTS WEIGHING LESS THAN 400 POUNDS AND HAVE A CENTER OF MASS LOCATED 4 FEET OR LESS ABOVE THE ADJACENT FLOOR OR ROOF LEVEL THAT DIRECTLY SUPPORT THE COMPONENT.
- COMPONENTS WEIGHING LESS THAN 20 POUNDS, OR IN THE CASE OF DISTRIBUTED SYSTEMS, LESS THAN 5 POUNDS PER FOOT, WHICH ARE SUSPENDED FROM A ROOF OR FLOOR OR HUNG FROM A WALL.

THE ANCHORAGE OF ALL ELECTRICAL COMPONENTS SHALL BE SUBJECT TO THE APPROVAL OF THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE OR STRUCTURAL ENGINEER DELEGATED RESPONSIBILITY AND ACCEPTANCE BY DSA. THE PROJECT INSPECTOR WILL VERIFY THAT ALL COMPONENTS AND EQUIPMENT HAVE BEEN ANCHORED IN ACCORDANCE WITH THE ABOVE REQUIREMENTS.

### ELECTRICAL DISTRIBUTION BRACING NOTES

THE ELECTRICAL DISTRIBUTION SYSTEM SHALL BE BRACED TO COMPLY WITH THE FORCES AND DISPLACEMENTS PRESCRIBED IN ASCE 7-16, SECTION 13.3 AS DEFINED IN ASCE 7-16, SECTIONS 13.6.5, 13.6.6, 13.6.7, 13.6.8, AND 2022 CBC, SECTIONS 1617A.1.24, 1617A.1.25, AND 1617A.1.26.

THE METHOD OF SHOWING BRACING AND ATTACHMENTS TO THE STRUCTURE FOR THE IDENTIFIED DISTRIBUTION SYSTEM ARE AS NOTED BELOW. WHEN BRACING AND ATTACHMENTS ARE BASED ON A PREAPPROVED INSTALLATION GUIDE (E.G., OSHD OPM FOR 2013 CBC OR LATER), COPIES OF THE BRACING SYSTEM INSTALLATION GUIDE OR MANUAL SHALL BE AVAILABLE ON THE JOBSITE PRIOR TO THE START OF AND DURING THE HANGING AND BRACING OF THE DISTRIBUTION SYSTEMS. THE STRUCTURAL ENGINEER OF RECORD SHALL VERIFY THE ADEQUACY OF THE STRUCTURE TO SUPPORT THE HANGER AND BRACE LOADS.

THE ELECTRICAL DISTRIBUTION SYSTEM IS DETAILED ON THE APPROVED DRAWINGS WITH SPECIFIC NOTES AND DETAILS.

### ELECTRICAL GENERAL NOTES

1. ALL WORK SHALL MEET THE LATEST ADOPTED ADDITIONS OF THE CALIFORNIA CODE OF REGULATIONS, TITLE 24 AND ALL OTHER APPLICABLE REGULATIONS, WHICH INCLUDE:

- CALIFORNIA BUILDING CODE 2022
- CALIFORNIA ELECTRICAL CODE 2022
- NON RESIDENTIAL CEC ENERGY STANDARDS 2022

2. NOTHING IN THE PLANS OR SPECIFICATIONS IS TO BE CONSTRUED TO PERMIT WORK NOT CONFORMING TO THESE CODES.

3. IT IS THE INTENTION OF THESE PLANS AND SPECIFICATIONS TO COVER EVERYTHING REQUIRED TO PROVIDE FOR COMPLETE AND OPERATIVE SYSTEMS. THE CONTRACTOR IS TO FURNISH LABOR, MATERIAL, TRANSPORTATION, EQUIPMENT, MISCELLANEOUS SERVICES, ETC. REQUIRED TO ACCOMPLISH THIS RESULT. ANYTHING WHICH MAY BE REASONABLY CONSTRUED AS A NECESSARY PART OF THE INSTALLATION IS TO BE INCLUDED, WHETHER OR NOT SPECIFICALLY SHOWN OR MENTIONED.

4. THE CONTRACTOR SHALL EXAMINE THE SITE AND EXISTING CONDITIONS AND MAKE ALLOWANCES IN THE BID FOR ANY CONDITIONS NOT SHOWN ON THE ELECTRICAL DOCUMENTS.

5. THE PLANS AND SPECIFICATIONS ARE INTENDED TO BE USED AS CONSTRUCTION GUIDELINES AND ARE NOT THE TOTAL INSTRUMENT OF CONTRACT DOCUMENTS. IT IS NOT THE INTENTION OF ANY CONSTRUCTION PLANS TO DIVIDE WORK AMONG DIFFERENT TRADES. VERIFY THE SCOPE OF WORK WITH THE ARCHITECT AND THE GENERAL CONTRACTOR.

6. ELECTRICAL ROUTING IS DIAGRAMMATIC ONLY. ACTUAL ROUTING & PHYSICAL CONDITIONS MAY VARY. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE ACTUAL ROUTING, CONNECTIONS, & PROVISION OF ALL APPURTENANCES NECESSARY FOR A COMPLETE & OPERATING SYSTEM.

7. ELECTRICAL EQUIPMENT SHALL HAVE AN APPROVED TESTING LABORATORY LABEL ATTACHED (UL, CSA ETC.) PER CEC 110.2.

8. PROVIDE LABELING AND DIRECTORIES FOR ALL SWITCHBOARDS AND PANELBOARDS PER CEC 408.4.

9. ELECTRICAL EQUIPMENT SHALL HAVE A SHORT CIRCUIT CURRENT RATING CAPABLE OF WITHSTANDING THE AVAILABLE SHORT CIRCUIT CURRENT PER CEC 110.9.

10. PROVIDE MINIMUM 30" WIDE x 78" HIGH x 36" DEEP [42" DEEP] WORK CLEARANCES IN FRONT OF PANELS, SERVICE OR EQUIPMENT RATED AT 120/208V 3Ø 4W [277/480V 3Ø 4W] PER CEC 110.26.

11. ELECTRICAL RECEPTACLE OUTLETS ON BRANCH CIRCUIT OF 30 AMPERES OR LESS AND COMMUNICATION SYSTEM RECEPTACLES SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), RECEPTACLES SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.

12. CONTROLS OR SWITCHES INTENDED TO BE USED BY THE OCCUPANT OF A ROOM OR AREA TO CONTROL LIGHTING AND RECEPTACLE OUTLETS, APPLIANCES, OR COOLING, HEATING AND VENTILATING EQUIPMENT SHALL BE MOUNTED BETWEEN 15" AFF AND 48" AFF AND SHALL COMPLY WITH CBC SECTION 11B-308. THE LOW REACH SHALL BE MEASURED TO THE BOTTOM OF THE OUTLET BOX AND THE HIGH REACH SHALL BE MEASURED TO THE TOP OF THE OUTLET BOX. IF THE REACH IS OBSTRUCTED (E.G. BY CASEWORK, COUNTERS, ETC.), SWITCHES AND CONTROLS SHALL BE LOCATED WITHIN THE REACH RANGES SPECIFIED IN CBC 11B-308.2.2 AND 11B-308.3.2.

13. ALL WALL AND SURFACE MOUNTED FIXTURES PROTRUDING IN THE PATH OF TRAVEL (POT) OR COMMON PEDESTRIAN WAYS SHALL COMPLY WITH CBC 11B-307.2, OR SHALL BE MOUNTED LESS THAN 27" AFF OR GREATER THAN 80" AFF, OR SHALL BE PROVIDED WITH A BARRIER CONFORMING TO CBC 11B-307.4.

14. WIRING FOR 120/208V AND 277/480V SYSTEMS SHALL BE MIN. #12 AWG THHN/THWN-2 COPPER.

15. FEEDERS SIZE #4 AND LARGER SHALL BE MEGGER TESTED. TEST RESULTS SHALL BE SUBMITTED TO THE ENGINEER.

16. COLORS/FINISHES/MATERIALS FOR ALL ELECTRICAL DEVICES, PLATES, LIGHT FIXTURES, ETC. SHALL BE CHOSEN BY THE ARCHITECT.

17. EXISTING EQUIPMENT TO BE REMOVED AND/OR REPLACED SHALL BE DELIVERED TO THE DISTRICT MAINTENANCE DEPARTMENT OR DISPOSED OF, AT THE DISCRETION OF THE DISTRICT.

18. CALL USA UNDERGROUND ALERT AND VERIFY WITH DISTRICT THE DESIRED ROUTING AND LOCATIONS OF UNDERGROUND CONDUITS AND STRUCTURES PRIOR TO TRENCHING.

19. ALL UNDERGROUND CONDUITS SHALL HAVE MINIMUM 2" COVER. TRENCH PER DETAIL 5E-1. INSTALL GALVANIZED RIGID STEEL RISERS & ELBOWS WHERE RISERS OCCUR. WRAP GRS BELOW GRADE OR PROVIDE PVC COATED GRS. EXPOSED CONDUIT SHALL BE GRS TO 8'-0", THEN ENT ABOVE AS APPROPRIATE. UNDER NO CIRCUMSTANCE SHALL PVC CONDUIT BE INSTALLED ABOVE GRADE.

20. CONDUIT INSTALLED ABOVE GRADE SHALL BE MIN. 3/4" TRADE SIZE. CONDUIT BELOW GRADE SHALL BE MIN. 1" TRADE SIZE.

21. INCLUDE ALL WORK REQUIRED TO INVESTIGATE, DEMOLISH, & RECONNECT EXISTING ITEMS.

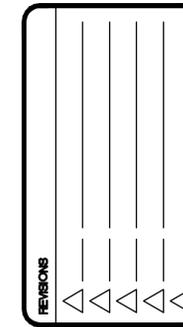
22. ALL LOW VOLTAGE EQUIPMENT SHALL BE DEENERGIZED PRIOR TO DEMO WORK. CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE TO LIVE EQUIPMENT.

### ELECTRICAL SYMBOLS

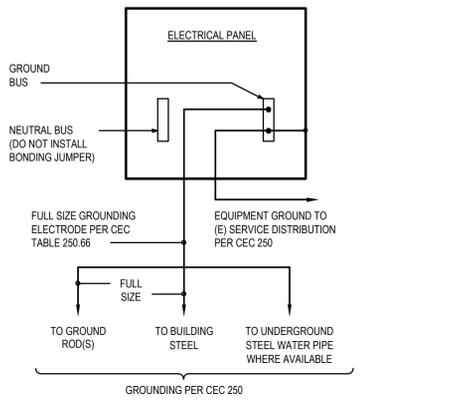
SYMBOL	DESCRIPTION	NOTES
	POLE WITH POST TOP AREA LUMINAIRE	
	POLE WITH AREA LUMINAIRE	
	RECESSED TROFFER LIGHT FIXTURE	
	SURFACE CEILING LIGHT FIXTURE	
	RECESSED DOWN LIGHT	
	WALL LIGHT	
	EXIT SIGN, CEILING	
	EXIT SIGN, WALL	AT +80" AFF
	EMERGENCY LIGHT FIXTURE	PROVIDE UNSWITCHED HOT CONDUCTOR TO BATTERIES
	SWITCHBOARD	REFER TO POWER SINGLE LINE DIAG.
	POWER PANEL	REFER TO PANEL SCHEDULE
	TERMINAL CABINET	
	DISCONNECT SWITCH, FUSIBLE, WP	REFER TO MECH. PLANS & SPECS.
	COMBO STARTER/DISCONNECT SWITCH, WP	REFER TO MECH. PLANS & SPECS.
	JUNCTION BOX	4-11/16" SQUARE BOX & COVER PLATE MIN.
	MOTOR	REFER TO MECH. PLANS AND SPECS.
	DUPLEX CONVENIENCE OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	DUPLEX CONVENIENCE OUTLET AT +18" AFF TO CENTER OF BOX, U.O.N.	20A SPEC. GRADE, NEMA GROUNDED
	GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	GFI DUPLEX OUTLET AT +18" AFF TO CENTER OF BOX, U.O.N.	20A SPEC. GRADE, NEMA GROUNDED
	WP, GFI DUPLEX OUTLET	20A SPEC. GRADE, NEMA GROUNDED
	WP, GFI DUPLEX OUTLET AT +18" AFF TO CENTER OF BOX, U.O.N.	20A SPEC. GRADE, NEMA GROUNDED
	DATA OUTLET (RJ-45 CAT6) WITH 2 JACKS	HOMERUN CABLES TO IDF.
	DATA OUTLET (RJ-45 CAT6) AT +18" AFF TO CENTER OF BOX, U.O.N.	2 BLUE JACKS & CABLES
	(2) WAP DATA JACKS (RJ-45 CAT6A)	HOMERUN CABLES TO IDF.
	MOUNTED IN ATTIC SPACE	2 YELLOW JACKS & CABLES
	WALL MOUNT 600V OUTLET (RJ-45 CAT6)	HOMERUN CABLES TO IDF.
	WALL MOUNT 600V OUTLET (RJ-45 CAT6) AT +45" AFF TO CENTER OF BOX, U.O.N.	1 WHITE JACK & CABLE
	WALL MOUNT DATA/COMM OUTLET	HOMERUN CABLES TO IDF.
	WALL MOUNT DATA/COMM OUTLET AT +18" AFF TO CENTER OF BOX, U.O.N.	2 BLUE AND 1 WHITE JACKS & CABLES
	WIREMOLD 5400 SURFACE WIREWAY	
	CEILING MOUNT PA SPEAKER	MATCH EXISTING SYSTEM COMPONENTS
	WALL MOUNT PA SPEAKER IN SURFACE ENCLOSURE	MATCH EXISTING SYSTEM COMPONENTS
	AUDIOVISUAL INPUT WITH HDMI/VGA/3.5MM AUDIO/USB JACKS AND WALL PLATE	INSTALL CABLES BETWEEN TEACHER STATION AND PROJECTOR. SEE DETAIL 6E-2.
	ANALOG CLOCK, BATTERY POWERED	VERIFY COMPATIBILITY WITH EXISTING SYSTEM
	MAIN DISTRIBUTION FRAME (MDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	INTERMEDIATE DISTRIBUTION FRAME (IDF)	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM TERMINAL BLOCK	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	P.A. SYSTEM HEAD END	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	TEL. SYSTEM TERMINAL BLOCK	WHERE EXISTING
	TEL. SYSTEM HEAD END	WHERE EXISTING
	FIBER OPTIC SPLICE LOCATION	SEE SCHOOL DISTRICT STANDARD SPECIFICATIONS
	CAT6 PATCH PANEL	WHERE EXISTING
	FIRE ALARM SLC & NAC TERMINAL BLOCKS	LOCATION FOR REFERENCE. SEE F.A. PLANS.
	FIRE ALARM EXPANDER PANEL	SEE F.A. PLANS
	FIRE ALARM CONTROL PANEL	SEE F.A. PLANS
	EMERGENCY VOICE/ALARM COMMUNICATION PANEL	SEE F.A. PLANS
	EXISTING WIRING TO REMAIN	
	WIRING BELOW GRADE	3/4" CONDUIT MIN.
	WIRING IN WALL OR CEILING	3/4" CONDUIT MIN.
	LOW VOLTAGE WIRING	
	CONDUIT RISER	3/4" CONDUIT MIN.
	FLEXIBLE CONDUIT	3/4" CONDUIT MIN.
	CONDUIT STUB AND CAP	3/4" CONDUIT MIN.
	HASH MARKS DENOTES QTY. OF CONDUCTORS	3/4" CONDUIT MIN.
	WIRE SIZE INDICATED, IF OTHER THAN #12 AWG	3/4" CONDUIT MIN.
	HOME RUN (TO PANEL 'A', CIRCUIT '15')	3/4" CONDUIT MIN.
	"EXISTING"	
	"UNLESS OTHERWISE NOTED"	
	"WEATHERPROOF" / NEMA 3R	
	"GROUND FAULT INTERRUPTER"	

DATE: 12/05/2023

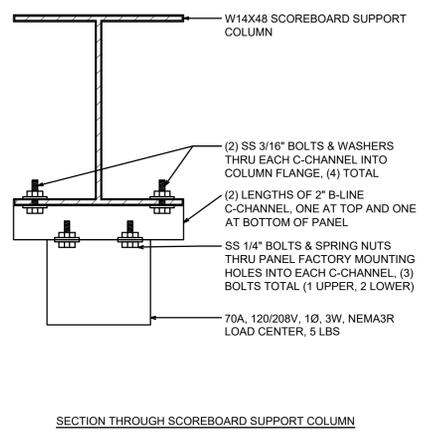
PARTIAL SITE PLAN  
MADERA HIGH SCHOOL  
BASEBALL SCOREBOARD  
MADERA, CA 93637



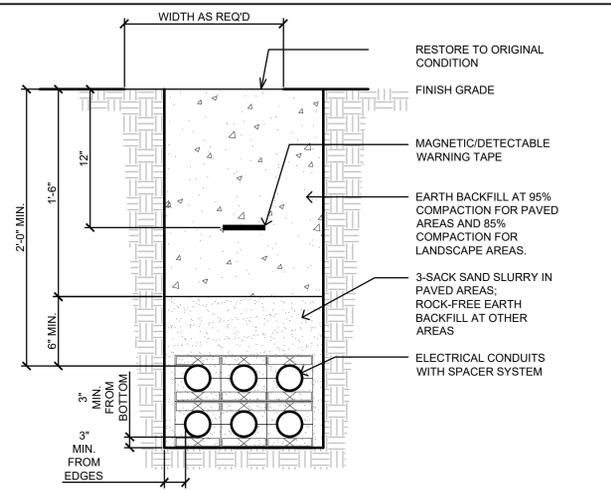
**Brooks Ransom ASSOCIATES**  
7415 N. PALM AVE. STE 100 | FRESNO, CA 93711  
(559) 449-8444 OFFICE | (559) 449-8404 FAX



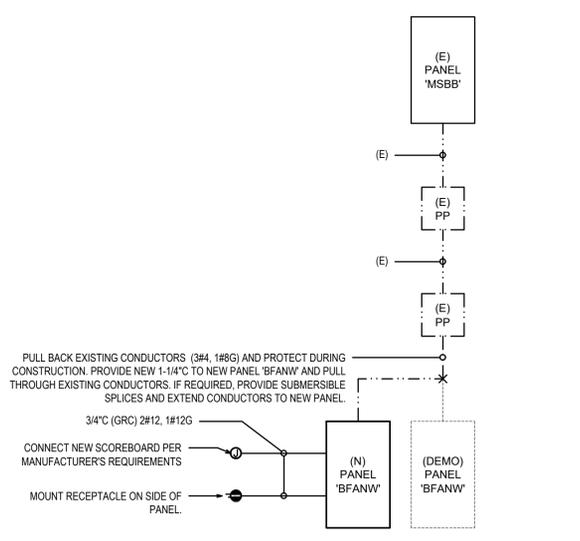
3 Panel Grounding Detail  
No Scale



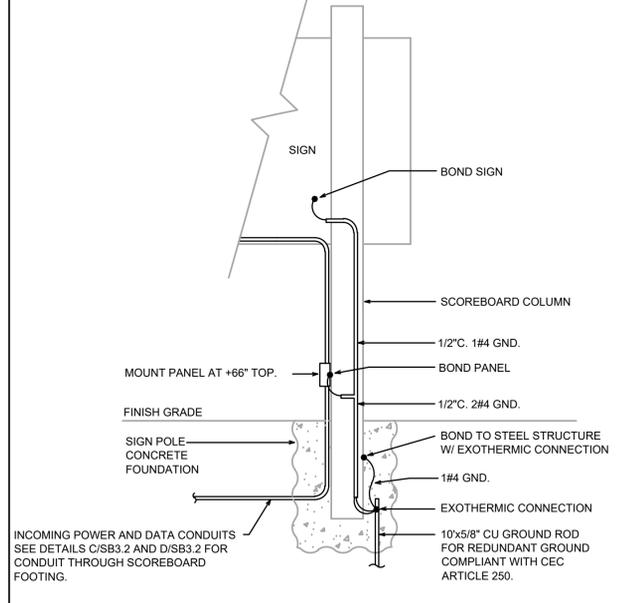
2 Panel Mounting Detail  
No Scale



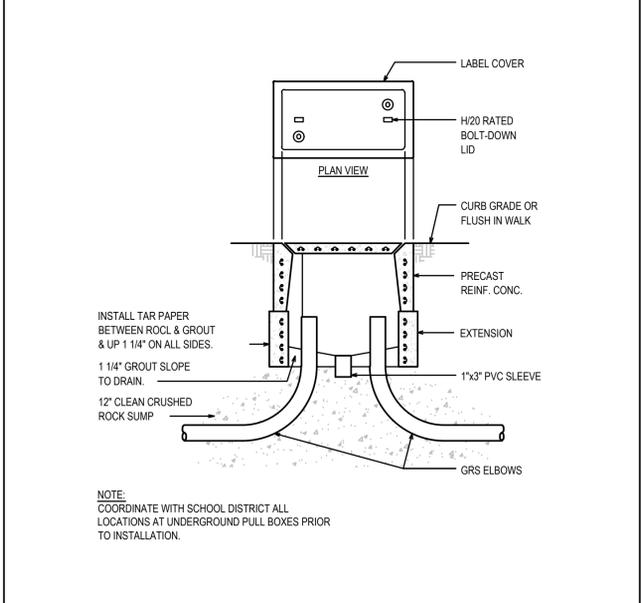
5 Trenching Detail  
No Scale



1 Single Line Diagram  
No Scale



4 Scoreboard Grounding Detail  
No Scale



6 Pull Box Detail  
No Scale

PROFESSIONAL ENGINEER  
CALIFORNIA REGISTERED PROFESSIONAL ENGINEER  
NO. 52386 EXPIRES 5-31-26  
CAYLOR R. RANDOM

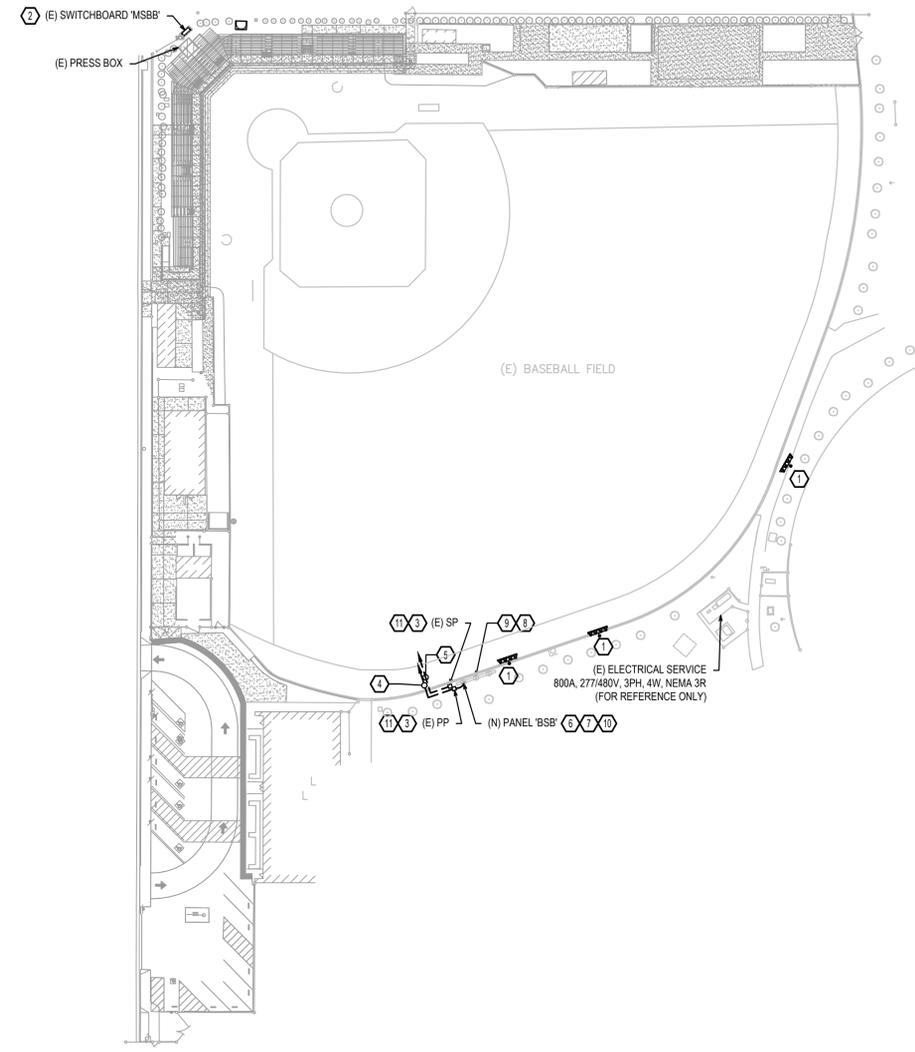
PROFESSIONAL ENGINEER  
CALIFORNIA REGISTERED PROFESSIONAL ENGINEER  
E17850  
SCOTT DAVIDSON

**HD**  
Hardin-Davidson  
Engineering  
356 Pollasky Ave.  
Suite 200  
Clovis, CA 93612  
559.323.4995 tel  
559.323.4928 fax

# KEY NOTES

- EXISTING SPORTS LIGHTING POLE TO REMAIN. SHOWN FOR REFERENCE. EXERCISE CAUTION WHEN EXCAVATING IN THE SCOREBOARD AREA FOR UNDERGROUND CONDUITS. PROVIDE LOCATION SERVICE OF PRIVATE CONDUITS. SPORTS LIGHTING IS FED FROM THE EXISTING ELECTRICAL SERVICE SOUTHEAST OF THE STADIUM.
- EXISTING MAIN SWITCHBOARD 'MSBB' TO REMAIN. 120/208V, 3Ø, 4W, 10KAIC, NEMA 3R.
- VERIFY EXISTING PULL BOX TO REMAIN. VERIFY LOCATION IN FIELD. IF PULLBOX IS NOT PRESENT, INTERCEPT EXISTING FEEDER AND/OR COMMUNICATION CONDUIT AND PROVIDE NEW 10X17 PULL BOX WITH 12" EXTENSION AND LID, H20-RATED. SEE DETAIL 6/E-1.
- EXISTING SCOREBOARD PANEL FEEDER TO REMAIN. 1-1/2"Ø, 3Ø, 1#ØG. EXTEND FEEDER IF REQUIRED. PROVIDE SUBMERSIBLE SPLICES.
- EXISTING SCOREBOARD DATA/CONTROL CONDUIT TO PRESS BOX. REPLACE EXISTING CONDUCTORS WITH NEW.
- DEMO EXISTING PANEL 'BBSB' FROM STEEL SCOREBOARD SUPPORT COLUMN. DISCONNECT EXISTING FEEDER AND PULL BACK TO PULL BOX PP-7 AND PRESERVE FOR RECONNECTION TO NEW PANEL.
- PROVIDE AND INSTALL NEW LOAD CENTER 'BBSB': 70A, 4 CKT, 120/208V, 1Ø, 10KAIC, NEMA 3R. S.Q.D #Q024L70RB (OR APPROVED EQUAL) PROVIDE (2) 20A/1P CIRCUIT BREAKERS. RECONNECT EXISTING CONDUCTORS. SEE DETAIL 1/E-1, 2/E-1, 3/E-1, 4/E-1, 5/E-1.
- DISCONNECT EXISTING SCOREBOARD POWER AND DATA/CONTROL. REMOVE EXISTING CONDUCTORS AND EXPOSED CONDUIT BACK TO NEAREST PULL BOX.
- POWER CONNECTION BY SCOREBOARD SUPPLIER. NEVCO MODEL #1609, 120V, 3.9A, 3/4"Ø, 2#12, 1#12G. SEE DETAIL 4/E-1.
- PROVIDE RECEPTACLE -42", 20A, 120V, GFCI-PROTECTED, WEATHER-RESISTANT RECEPTACLE WITH WEATHERPROOF WHILE-IN-USE COVER. 3/4"Ø, 2#12, 1#12G. SEE SINGLE LINE DIAGRAM 1/E-1.
- RELOCATE/REPLACE EXISTING PULL BOXES IF REQUIRED. SEE DETAIL 6/E-1. IF REPLACING BOXES, REPLACE WITH LIKE SIZE, H20-RATED.

IDENTIFICATION STAMP  
DIV. OF THE STATE ARCHITECT  
APP: 02-122088 INC:  
REVIEWED FOR  
SS  FLS  ACS   
DATE: 04/02/2024



**PARTIAL SITE PLAN**  
MADERA HIGH SCHOOL  
**BASEBALL SCOREBOARD**  
MADERA, CA 93637

REVISIONS


**Brooks Ransom**  
ASSOCIATES  
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(559) 449-8444 OFFICE | (559) 449-8404 FAX

**1 PARTIAL ELECTRICAL SITE PLAN**

SCALE: 1" = 50'-0"

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Clovis, CA 93612  
559.323.4995 tel  
559.323.4928 fax

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA  
NO. 52386  
EXPIRES 5-31-26  
CAYLOR R. RANSOM

REGISTERED PROFESSIONAL ENGINEER  
CALIFORNIA  
E17850  
SCOTT DAVIDSON

SHEET:  
**E-2**  
PROJECT: 23314

18 March 2024 9:32 AM Z:\Clients\BrooksRansom Associates\23134 - Madera USD Scoreboards (MHS, MSHS)\CAD Files\23134 - Madera Baseball - Electrical - v3.dwg patrick may